

UBC Social Ecological Economic Development Studies (SEEDS) Sustainability Program

Student Research Report

Redesign of Wesbrook Mall and Chancellor Boulevard Final Design Report

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University of British Columbia

CIVL 446

April 7, 2017

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April 7th, 2017

Krista Falkner, P.Eng., M.A.Sc
Transportation Engineer
UBC Campus and Community Planning
2210 West Mall
Vancouver, BC
V6T 1Z4

Dear Ms. Falkner,

Re: CIVL 446 Project II - Redesign of Chancellor Boulevard / Wesbrook Mall Intersection at UBC

We are pleased to be given the opportunity to redesign the Chancellor Boulevard - Wesbrook Mall intersection. Please see attached for the final design report titled *Redesign of Wesbrook Mall and Chancellor Boulevard*.

This design expands on the recommended option discussed in the summary report submitted on March 3rd, 2017. Included in this report are detailed design drawings, final schedule, and Class A cost estimate.

The intersection is recommended to be signalized and includes bike lanes in all travel directions, as well as crosswalks and pedestrian signals. Construction for this project begins on May 1st, 2017 and concludes on July 19th, 2017. The cost for implementation is \$899,341.90.

If you have any questions or concerns, please contact our team representative, Chi-Yan Leung. We look forward to hearing back from you soon.

Sincerely,

Team 12
Enclosure, Final Design Report

EXECUTIVE SUMMARY

The Wesbrook Mall-Chancellor Boulevard intersection is a key point of entry located at the north end of the University of British Columbia (UBC) Vancouver campus. The atypical configuration of the current intersection results in congestion during peak hours and creates unsafe conditions for road users, such as cyclists and pedestrians. The main project objective is to provide a detailed design of a safe and efficient intersection, in addition to meeting the criteria and goals requested by the client. Furthermore, additional objectives include accommodating future traffic volumes that are projected to the year 2041, minimizing project costs, integrating sustainable practices per UBC's Campus Plan, and developing an appealing gateway to UBC.

Technical consideration and constraints include property line locations, underground utilities, geotechnical obstacles, and other technical requirements outlined by federal and provincial codes and standards. An annual growth rate of 2.2% was considered in the projection of future traffic volumes based on UBC's enrollment rate through the past ten years. In lieu of WB-17 trucks, a demo truck was used for the analysis on AutoTurn to ensure that the design is sufficient for truck turning movements. An analysis was conducted on Synchro and SimTraffic to determine the most feasible configuration. Roundabout, stop-controlled, and signalized configurations were analyzed. It was determined that a signalized intersection is the most feasible option that can adequately accommodate the projected future traffic volume in 2041; thus, it was selected for the preliminary design.

Notable features of the design comprise of a signalized configuration with one through/right turn lane in the eastbound direction, one through lane, one through/left turn lane and one left turn lane in the westbound direction, and one left turn/right turn lane in the northbound direction. In addition to the vehicular lanes, the proposed intersection design also includes bike lanes in all travel directions, crosswalks, and traffic and pedestrian/bike signals.

The proposed campus gateway features a concrete archway with the letters "UBC" engraved on a 0.5 m thick steel plate in the center of the gateway. The structure is located on the median on the west side of the intersection, spanning the entire median. The structure is 5m wide, and 8m high. The columns of the arch will be 305x305x13 HSS columns, and will be placed on 1091 x 1091 mm square footings, which are located 1 m below grade.

Geotechnical and structural analysis show that the structure is able to adequately resist design loadings from dead, snow, wind, and earthquake loads.

Construction commences on May 1st, 2017 and finishes in July 19th, 2017. 78 working days are required to complete the project. A working week consists of 7 days, from Monday to Sunday, with the exception of Canadian statutory holidays.

The total cost to implement the selected design is approximately \$899k including 6% contingency plus 5% GST. \$460k of the total cost goes to purchase of the traffic lights. Funding for the project is from UBC. Additional funding can be acquired from BikeBC and New Building Canada Fund.

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1.0 INTRODUCTION

UBC has awarded Team 12 with the Redesign of Wesbrook Mall and Chancellor Boulevard. The project consists of improving the intersection as well as designing a structural gateway into the campus. The purpose of this report is to summarize the final design of the intersection, and will provide all necessary information for the client to proceed with tendering. Such information includes detailed design drawings, schedule, and cost estimate.

The project intersection is a major entrance to UBC. Located at the north end of campus near the Chan Center and Museum of Anthropology, the route can be accessed through West 4th Avenue.

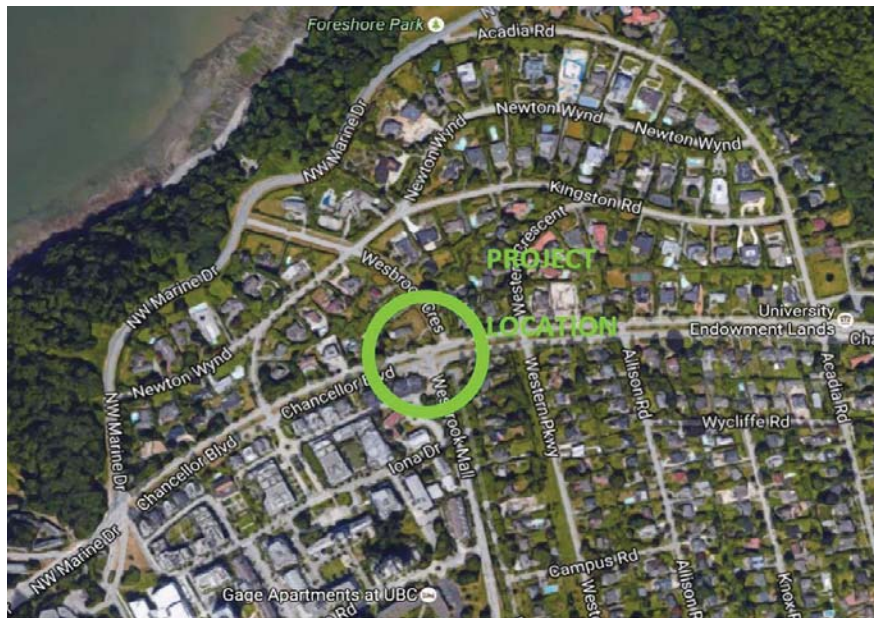


FIGURE 1 PROJECT LOCATION

Prior to the submission of the final design report, previous submissions to the client include a conceptual report (October 24, 2016), a preliminary report (November 28, 2016), followed by a summary report (March 3, 2017).

Information in these documents include:

- Decision making process for the intersection
- Stakeholder engagement plan
- Regulations, such as required permits, safety and environmental bylaws
- Previous design costs

Member contributions are summarized in the table below.

TABLE 1 MEMBER CONTRIBUTIONS

Member	Contribution
<i>Celine Au</i>	Transportation detailed design Drafting detailed plan drawings
<i>Cliff Chun</i>	Structural design checks Standard and project review
<i>Chi Yan Leung</i>	Structural detailed design Drafting transportation cross sections
<i>Yu Feng (York) Liao</i>	Scheduling Construction and traffic management plan
<i>Mandy Tam</i>	Structural detailed design Drafting structural drawings
<i>Jiahua (Evan) Yan</i>	Cost estimate Drafting standards detail (i.e. typical curb basin, curb and gutter)

2.0 DESCRIPTION OF DESIGN

A full detailed drawing set is included in Appendix A. A summary description of the design are below.

2.1 TRANSPORTATION DESIGN

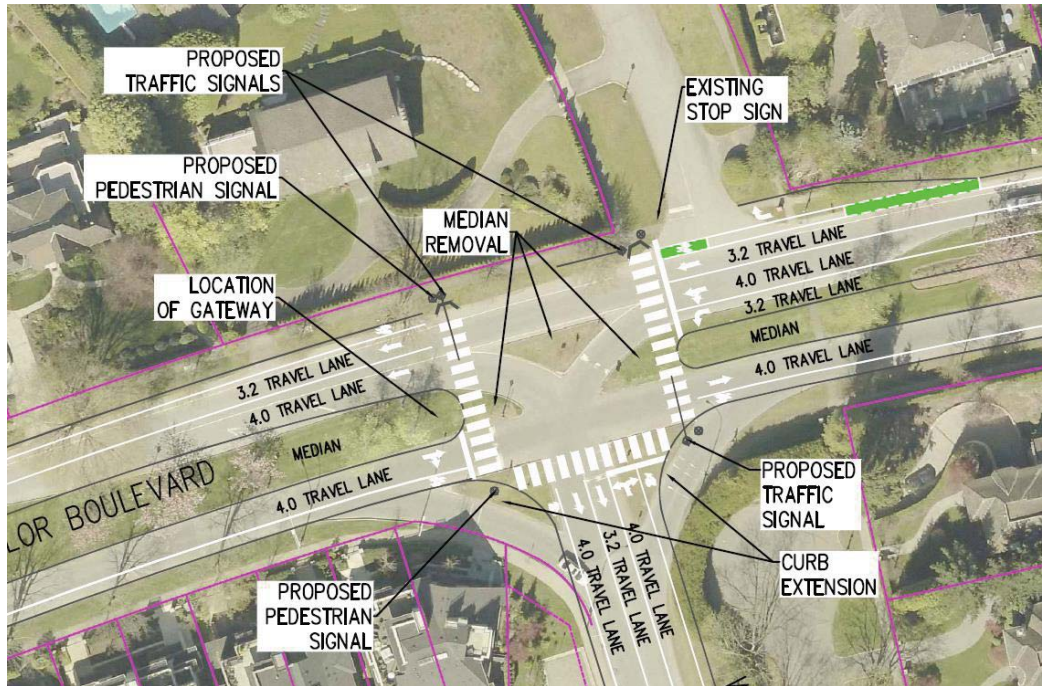


FIGURE 2 DETAILED DESIGN PLAN VIEW

The intersection will be improved to be a signalized intersection. Key benefits include increased safety for all users, increased clarity for drivers, and allows for flexibility in design. A signalized intersection was selected in the early design phases during the Synchro analysis, as it was the only option to be able to sustain the future traffic volumes. Other tested intersections included a roundabout and stop intersection.

Eastbound consists of one through/right travel lane. Westbound consists of one through lane, one through/left lane, and one left lane. Northbound consists of one left/right travel lane. Other components include bike lanes in all travel directions, crosswalks, traffic signals, and pedestrian signals. Pavement markings include: paint to delineate lanes, bicycle lane markers, turning arrows, and green surface treatment to highlight conflict areas with cyclists. Above ground, 4 light posts and 1 curb basin will need to be moved.

Below ground, the areas will be highlighted for contractors to be aware of underground utilities when removing medians.

2.2 STRUCTURAL DESIGN

The structural design was completed in accordance with the following codes and standards: British Columbia Building Code 2012, CSA A23.3 (Concrete Design Standard) and CSA S16 (Steel Design Standard).

For the structural gateway, design loads tested include: dead, snow, wind, and earthquake. The design life is considered to be 25 years to match the transportation analysis. Specified loadings can be found in the summary table below. Refer to Appendix E for detailed calculations of each design load.

TABLE 2 SUMMARY OF SPECIFIED LOADS

Specified Load	Dead	Snow	Wind	Earthquake
Loading	230.56 kN	2.12 kPa	0.51 kPa	52.11 kN

The gateway is treated as a single storey frame under "other systems" for the seismic-force resisting system. Due to R_d exceeding 1.5, the upper bound of V will be used as the minimum lateral earthquake force. The structure's weight is set as 1.0kN to obtain the seismic loading as a percentage of the weight. Additionally, the minimum factor of safety used for sliding and overturning is 1.5 The figure below shows the arched gateway that will be constructed on the West leg of the intersection.

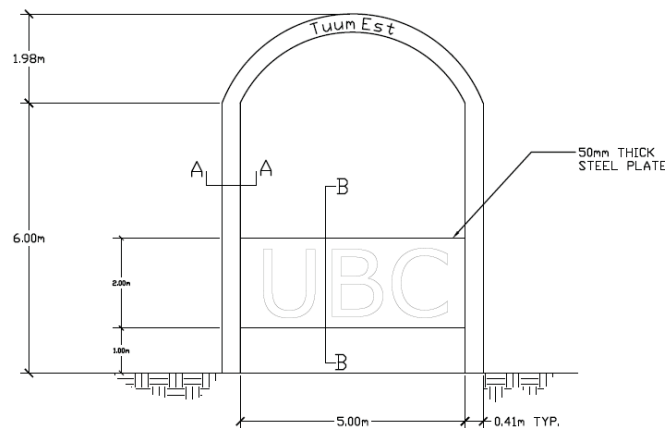


FIGURE 3 STRUCTURAL GATEWAY

Detailed drawings of the footing and specific section cuts can be found in Appendix E.

The gateway will be composed of a concrete arch with UBC's motto of *Tuum Est* engraved on it. The structural system of the arch is two 305x305x13 HSS column on each side with the concrete arch placed on top and steel plates with the letters UBC engraved placed in between the two columns. A minimum concrete cover of 50mm will be applied around the columns to prevent the HSS from being exposed to the elements.

For the UBC sign, two 50mm steel plates will be used. Five W310x375 steel beams will be placed between the steel plates to provide reinforcement and ensure deflections caused by wind loading is acceptable. The steel beams are welded to the HSS using fillet welds before the placing of the concrete cover. The steel plates are then bolted to the top W-shape for load transfer from the plates to the beams to the column. The remaining steel beams are to prevent the steel plates from deflecting under strong wind loads and are not gravity load bearing elements.

Two 1091 x 1091 mm square footings will be placed 1 m below grade. The size of the footings do not require shear reinforcement to be designed. The flexural reinforcement consists of 10-15M rebars spaced evenly in each direction, with a 75 mm cover on all sides.

Detailed design calculations and design drawings can be found in Appendix E.

3.0 DESIGN CRITERIA

3.1 TRANSPORTATION DESIGN

3.1.1 ALIGNMENT AND LANE WIDTH

Chapter 2.1 Alignment and Lane Configuration of the TAC Geometric Design Guide for Canadian Roads has been taken into account while designing for the intersection for this project. In addition, the lane widths for roads in all directions follow the design requirements that is suitable for WB-17 trucks

3.1.2 AUTOTURN ANALYSIS

AutoTurn was used in the design configuration to ensure WB-17 trucks are able to make turns. The relevant drawing is attached to Appendix A: Drawing T-07.

3.1.3 TRAFFIC FLOW PREDICTION

The team has analyzed the future traffic load in both 10 years and 25 years. Traffic is projected using a 2.2% annual average increase based on the enrollment numbers UBC provided from the past ten years. The only adjustment needed for the intersection is to optimize signal timing to complement the growth of the flow.

3.2 STRUCTURAL DESIGN

Design loads such as dead load, snow load, wind load, and earthquake load is contained while designing for the structural component of the project. British Columbia Building Code 2012, CSA A23.3 (Concrete Design Standard) and CSA S16 (Steel Design Standard) were followed for the structural design. A reasonable factor of safety (1.5 for sliding and overturning) was ensured throughout the design process.

4.0 STANDARDS AND SOFTWARE PACKAGES

4.1 TRANSPORTATION DESIGN

To model the intersection, Synchro 6 and SimTraffic were used. Standards include:

- TAC Geometric Design Guide for Canadian Roads
- AASHTO Design and Controls Criteria
- BCMOTI Manual of Standard Signs and Pavement Markings
- MUTCD
- AASHTO Guide for Design of Pavement Structures

4.2 STRUCTURAL DESIGN

Standards for the structural component include:

- NBC 2010
- NBC 2010 Structural Commentaries
- CSA A23.3-04
- CSA S16-09

NBC 2010 and NBC 2010 Structural Commentaries were used to determine structural loading as the content reflects those outlined in BCBC 2012. Although NBC 2015 is available, there is no BC version to accompany the new edition.

4.3 COST ESTIMATION AND PROJECT SCHEDULING

Cost estimation was mostly done on Microsoft Excel with help of a template. Microsoft Project was used to plan and present the schedule of the project.

5.0 TECHNICAL CONSIDERATIONS

5.1 TRANSPORTATION DESIGN

The road design is above ground, therefore sanitary, water supply, storm sewage and electrical utilities underneath the intersection will not be affected. During construction activities for the structural gateway, locations and depths of underground utilities are well denoted for the contractors to stay clear. Should there be any utility work required, UBC Energy and Water Service will be contacted to perform the work.

Re-routing has been designed to accommodate vehicles and transit passing through the intersection. Traffic operations will be maintained during all three construction phases. Property Lines are respected and thus both the design and the construction will not interfere with private properties.

The geological composition of the area was determined through drilled wells. The ground surface consists of glacial till, followed by alternating units of silt and Quadra sand as depth increases. Composition of the till is of dense sands, silt and clay in addition to some boulders. According to the geotechnical reports, the soil composition and condition are adequate for the design and the construction of both the intersection and the structural gateway.

Traffic volume was projected using an average annual increase rate of 2.2% based on enrollment data from the past ten years.

Road structure, curb and sidewalk designs are based on City of Surrey construction standard drawings, which are attached in Appendix A.

5.2 STRUCTURAL DESIGN

The foundation design considered quadra sand as the soil. The soil was assumed to have a unit weight of 19.8 kN/m³. Using Meyerhof analysis for bearing capacity of soil underneath the footing, the Factor of Safety was calculated to be 7.5. After computing the additional stresses due to bending, it was determined that stresses due to overturning moment are small compared to the axial loading, thus the bearing capacity of the soil under the footing is sufficient.

The footings are designed with a cover of 75 mm due to the exposure class in accordance with CSA A.23.3. Care should be taken to consider the utilities under the soil during excavation and construction to ensure that no underground utilities are disturbed.

6.0 CONSTRUCTION

6.1 DRAFT PLAN

The Chancellor Boulevard and Wesbrook Mall intersection improvement project sequence are presented in the figure below, some of the key construction requirements of the project include:

- Compliance with WorkSafe BC Standards
- Construction to be complete within a specific timeline
- Adherence to UBC Campus Planning guideline and sustainable goals
- Quality of work must meet the design standards outlined in Section 4.0
- Abide by the regulations set by City of Vancouver
- Continued public access through intersection during construction phase

6.2 ANTICIPATED ISSUES

The following sections outline the various issues that may arise during construction, in addition to the measures taken to mitigate or minimize the impact of the issue.

6.2.1 TRAFFIC MANAGEMENT

During the construction phase, various legs of the intersection will become inaccessible to road users. Hence, the need for traffic control to direct the flow of vehicles and minimize impact of the construction. Team 12 has developed a detailed traffic management plan with detour routes for vehicles and public transit.

Construction is to take place during the summer at UBC, thus it is expected that there will be less demand for the intersection. Construction will be executed in phases such that the intersection will remain accessible to vehicles.

The phases are outlined in TABLE 3 below. Contractors are responsible for hiring traffic control personnel or flaggers to assist road users passing through the construction area. The contractor is to refer to UBC's Traffic

Management Plan Guidelines for Construction and adhere to the guidelines stipulated. Drawings of signage for each phase can be found in Appendix A.

TABLE 3 TRAFFIC MANAGEMENT PHASES

Traffic Management Phases	Estimated Time (Days)	Activities	Respective Drawing
1	13	<ul style="list-style-type: none"> Widen Wesbrook Mall Traffic signals & pedestrian flasher installation Wesbrook Mall Pedestrian Sidewalk Construction Curb extension 	T-12 T-13 T-14
2	23	<ul style="list-style-type: none"> Widen Chancellor Boulevard West Leg Gateway Construction 	T-15
3	17	<ul style="list-style-type: none"> Widen Chancellor Boulevard East Leg 	T-16

The affected bus routes include C18, C20, 44, and 84. Buses are to detour along the specified routes to decrease travel time variability for transit and decrease the demand on the intersection. All bus routes will travel along Wesbrook Mall, University Boulevard and Allison Road. The C18 and C20, in particular, will travel along NW Marine Drive to reach their regular stops. FIGURE 4 and FIGURE 5 illustrate the detours buses will be taking during construction. Regular service will resume upon completion of the project.

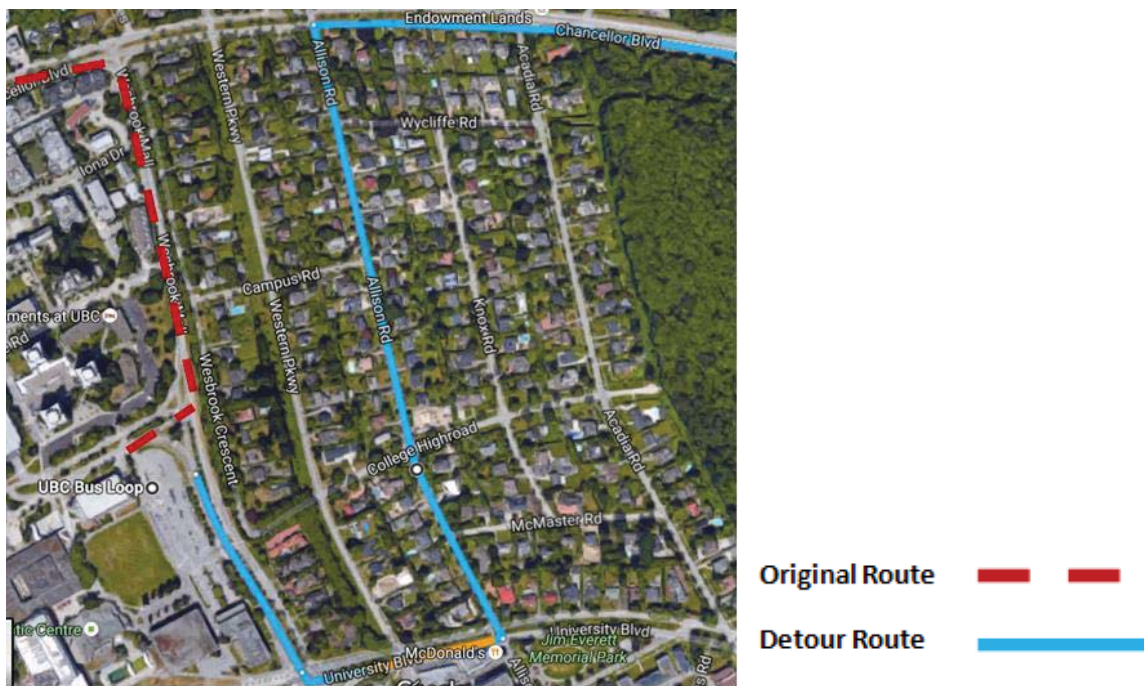


FIGURE 4 PROPOSED DETOUR FOR 44, AND 84

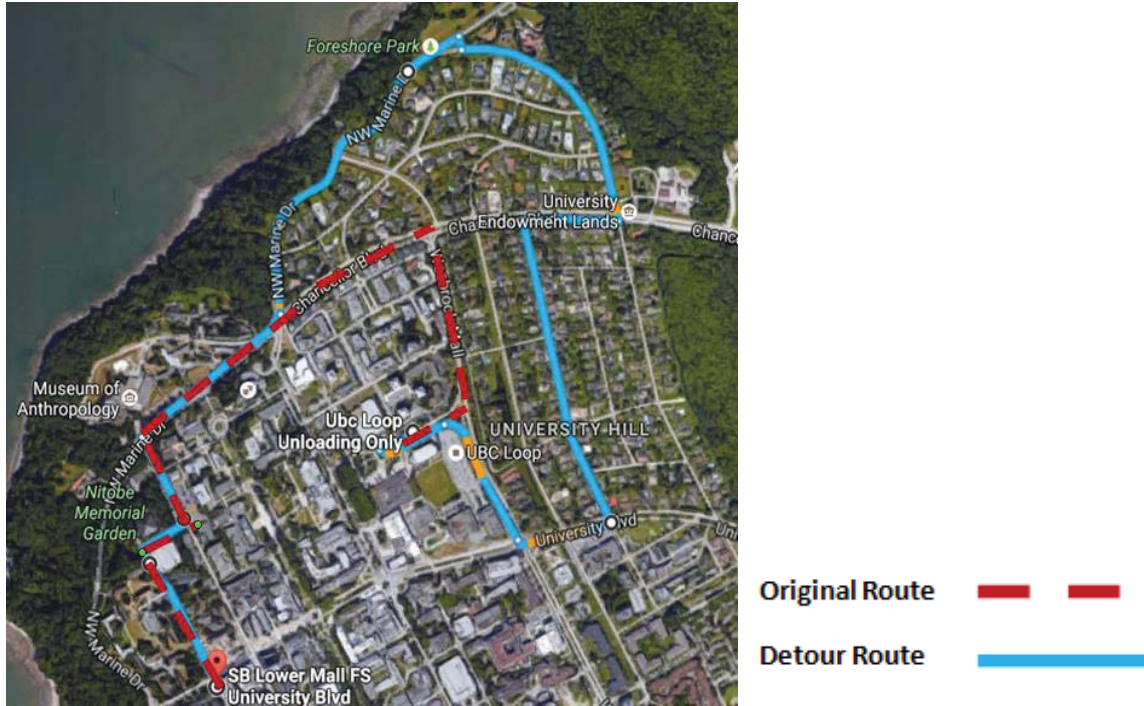


FIGURE 5 PROPOSED DETOUR FOR C18 AND C20

6.2.2. NOISE LEVEL AND POLLUTION

Following Vancouver’s Noise Control Bylaw 6555, street construction will only be carried out during the following times:

- Monday to Saturday from 7:00AM to 8:00PM
- Sundays and holidays from 10:00AM to 8:00PM

Dust emitted from the site will be controlled with adequate water or dust suppressant. These guidelines will be followed during the construction phase to reduce disturbance to the neighborhood.

6.2.3. PLACEMENT OF CONSTRUCTION MATERIAL AND EQUIPMENT

The placement of material and equipment will be analyzed in conjunction with the construction timeline to efficiently use the space available of the surrounding area. Disturbance to the neighborhood will be minimized.

6.2.4. POTENTIAL REMOVAL OR RELOCATION OF UNDERGROUND UTILITIES

The existing location of the utilities has been analyzed with site work and do not require relocation at this point in time. Section 5.1 provides more details regarding utility planning during construction.

6.2.5. LOST TIME INCIDENCE

The construction schedule provides sufficient float and flexibility such that proceeding tasks and the project schedule are not delayed. In the event of an injury, it is the responsibility of the contractor to ensure that the work carried out is in compliance with WorkSafe BC standards. A Construction Safety Officer is to be on site at all times with a first aid and CPR certification.

6.2.6. CONSTRUCTABILITY ISSUES

Unexpected changes in site conditions or incidence may affect the constructability of some components. In the event of an issue, the contractor is to notify and consult the respective designer to confirm that any changes required to construct the item on site are acceptable and adhere to standards. Construction may not proceed for designed components unless an engineer's approval is recorded on a written document.

7.0 SCHEDULE

The Wesbrook Mall-Chancellor Boulevard intersection project schedule has been divided into two major phases, design and construction with durations of 225 and 78 working days respectively. A working week consists of 7 days from Monday to Sunday. In order to minimize construction impact in the neighbourhood, the construction is to occur from 8 AM to 5 PM, with the exception of Sundays as noted in Section 6.2.2, to condense the construction period. No work activities on the project are required during any Canadian statutory holidays.

The schedule presents stakeholders and general contractors with the construction concept. Project schedules may undergo changes during the construction period depending on factors such as a change in scope or change in contractors. The project schedule forecasting the implementation date has been developed based on precedent examples with similar scope. The schedule is located in Appendix C in the form of Gantt charts.

The following presents the highlights of the project schedule:

- Proposal, Conceptual Design and Preliminary Design activities are restated in the preliminary schedule which consists of 74 working days
- Detail Design Phase for the intersection and gateway occurred over a 3 month window.
- Acquisition of permits proceeded after detailed designs were approved:
 - 40 working days to acquire BC Ministry of Transportation Permit
 - 30 working days to acquire UBC Campus Plan Permit
- Once the permitting phase was complete, a 33-day window was used to finalize the design in the Final Design Phase
- Procurement Phase for tendering is estimated to be a duration of 38 days
- Project construction is scheduled to begin on May 1st, 2017, with an estimated 78 working days for the Construction Phase to be complete
- The intersection aims to be available for the public on July 19th, 2017.

FIGURE 7 on the following page illustrates the key milestones throughout the project timeline.

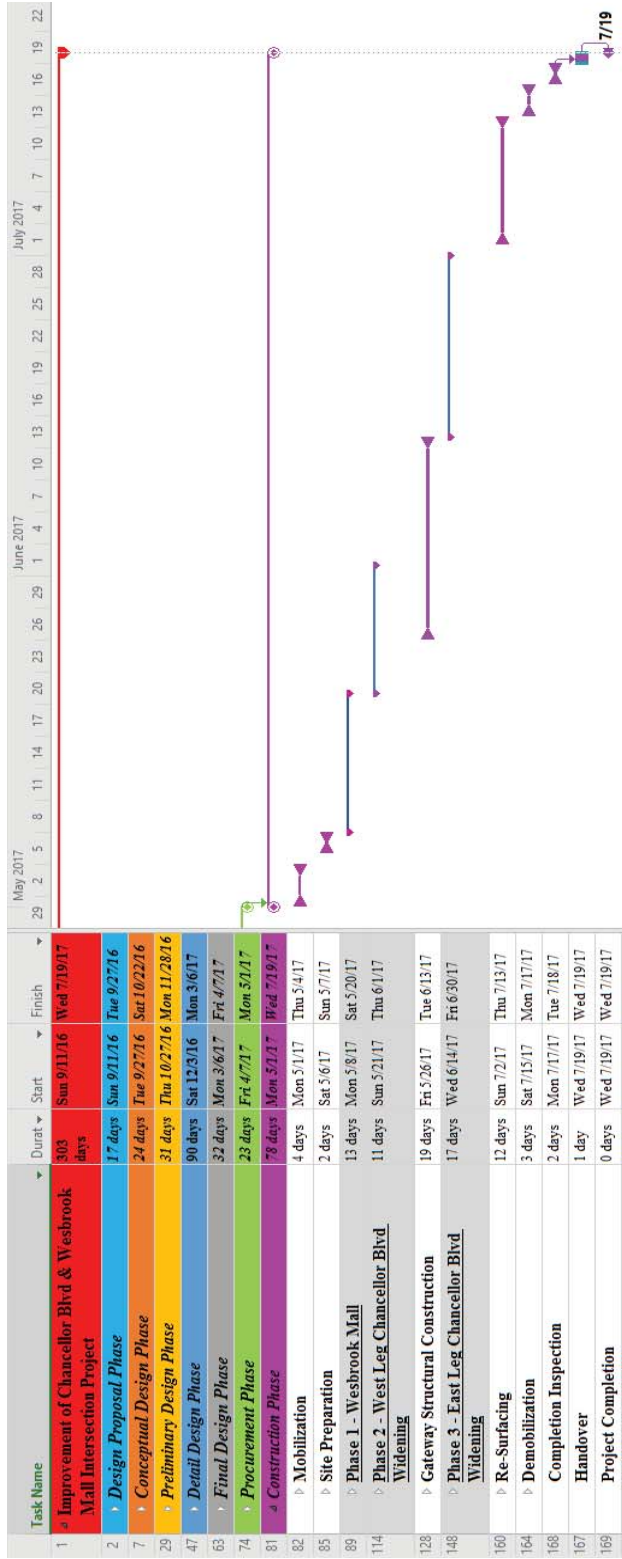


FIGURE 6 CONDENSED SCHEDULE

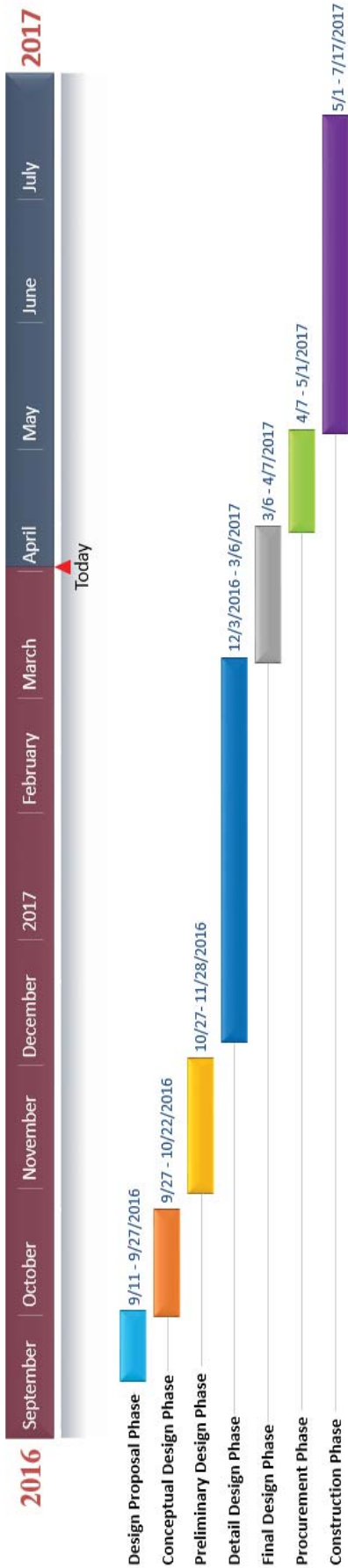


FIGURE 7 PROJECT TIMELINE

8.0 COST ESTIMATE

A detailed cost estimate breakdown is in Appendix B. Key values are discussed in the sections below.

8.1 PROCUREMENT COST

Procurement phase costs \$11.5k. Team members that will be working on the project are five EIT's and a senior project manager, with daily professional service rates of \$968 and \$1560, respectively.

8.2 CONSTRUCTION COST

A general breakdown of construction costs is presented below. This includes costs such as labour cost, equipment cost and material cost. Administrative costs such as permitting, obtaining licenses and other required paperwork are also included.

TABLE 4 CONSTRUCTION COST ESTIMATE

Construction Phases	Cost
Mobilization	\$8700
Site Preparation	\$12400
Phase 1	\$488779
Phase 2	\$26683
Structural Gateway Construction	\$6248
Phase 3	\$35969
Re-surfacing	\$46868
Demobilization	\$5000
Completion/Delivery	\$1380
General Contractor Misc Cost	\$182255
Subtotal	\$ 808,034.0
6% Contingency	\$ 48,482.0
Total	\$ 856,516.1
5% GST	\$ 42,825.8
Final	\$ 899,341.9

8.2.1 EARTHWORKS

Total area that needs to be excavated and backfilled is about 1100m². Assuming a general depth of 0.55m and a bulk factor of 120%, excavation volume is around 700m³ including sloping for easy access and safety. For backfill, a factor of 115% is used to account for waste and compaction. Productivities and unit rates for labor, materials and equipment can be found in Appendix B.

8.2.2 CURB AND GUTTER

Some sections of curb need to be re-installed due to extension of curb or removal of old curb. Total length of curb and gutter required to install during all three phases is 267m with a unit price of \$70 per meter.

8.2.3 MARKINGS

New lane lines that needs to be painted is about 160 meter long and the unit price for the paint job is \$80 per meter. Hence, the total cost of new lane lines is approximately \$12,800. In addition, the unit price for 16 lane markings, including bicycle lane markings and turning markings, is \$180 each which leads to a total cost of \$2,880. Finally, stood-up signs that indicate bike lanes should be installed in each direction. A total number of 6 signs are required for the project and the unit price is \$300, giving the total cost of \$1,800.

8.2.4 PERMITS

With the combination of both UBC and the BCMOTI required permits, the total cost of permit is \$6,000. Permit regulations are following:

- UBC Planning Development & Building Regulations
- UBC Planning Plumbing/Sprinkler Permit Fee Schedule
- UBC Planning Plumbing/Sprinkler Trade Permits

The application fee for permit will be a one-time cost and the detailed breakdown of the components of the permits are provided in Appendix B.

8.2.5 TRAFFIC LIGHTS

The initial cost for traffic light is approximately \$153,000 per light including installation. This project will require a total number of three traffic lights in three different intersections. The cost per traffic light comprises of:

- The traffic signal controller
- Sensors for vehicle detection
- Pedestrian push button systems

- Underground concrete bases for pole support
- Signal poles and arms
- Conduit and wiring between the controller and all signal equipment
- Data communications to the traffic management center

8.2.6 TREE REMOVAL

A total of 8 trees are to be removed. Three of which are located on the median that will be removed. Another two will be cut to make room for the construction of the gateway design. The last three trees are to be removed to promote safety by allowing the drivers to see the traffic lights properly. The labor cost for tree removal is \$750 per tree and the total cost is \$6,000.

8.3 OPERATING AND MAINTENANCE COSTS

All lane markings are required to be repainted every 10 years. The average maintenance costs for lane paints and road marking are \$4,488 and \$306 per year. Additionally, it costs around \$8000 a year to operate and maintain a traffic light. Operations and maintenance include electric bills, routine maintenance as well as optimization of traffic signal phasing. Annual operation and maintenance expenses are specified below.

TABLE 5 MAINTENANCE COST ESTIMATE

Operating/Maintenance	\$24,000.0	per yr	lights
Maintenance	\$4,488.0	per yr	lane marking
	\$306.0	per yr	Road Marking

Additional information regarding to detailed cost breakdowns, construction activities involved, quantities, unit rates, operating cost, maintenance cost and sample calculations are in Appendix B.

9.0 CONCLUSION

Team 12's design of a signalized intersection is one that meets all of the project objectives and needs of the community. With the inclusion of bike lanes and pedestrian crosswalks, this design not only increases safety but promotes the use of active transportation in the area. Furthermore, the design will be able to sufficiently sustain traffic volumes up until 2041.

The detailed design presented in this report is feasible at a cost of \$899341.90, and can be efficiently constructed within the project time frame.

The structural arch will become a prominent gateway to the campus. While providing a unique aesthetic appeal to the neighbouring community, the arch will also become a strong symbol of UBC that will be easily recognized.

APPENDIX A : CAD DRAWINGS

REDESIGN OF WESBROOK MALL AND CHANCELLOR BOULEVARD DRAWING SET

TEAM 12
APRIL 7, 2017

CLIENT : UBC SEEDS

SHEETS

- T-01 Detailed Design Drawing
- T-02 Geometric Plan Drawing
- T-03 Pavement Markings Plan Drawing
- T-04 Underground Utilities Plan Drawing
- T-05 Median and Pavement Removal Plan Drawing
- T-06 Curb Extension Plan Drawing
- T-07 AutoTurn Check
- T-08 East Section View
- T-09 South Section View
- T-10 West Section View
- T-11 City of Surrey References
- T-12 Phase 1 Traffic General Drawing
- T-13 Phase 1 Traffic Management sheet 1
- T-14 Phase 1 Traffic Management sheet 2
- T-15 Phase 2 Traffic Management
- T-16 Phase 3 Traffic Management
- S-01 Structural Arch
- S-02 Structural Details
- S-03 Footing Details



General Notes

No.	Revisions/Issues	Date

Prepared By: CA

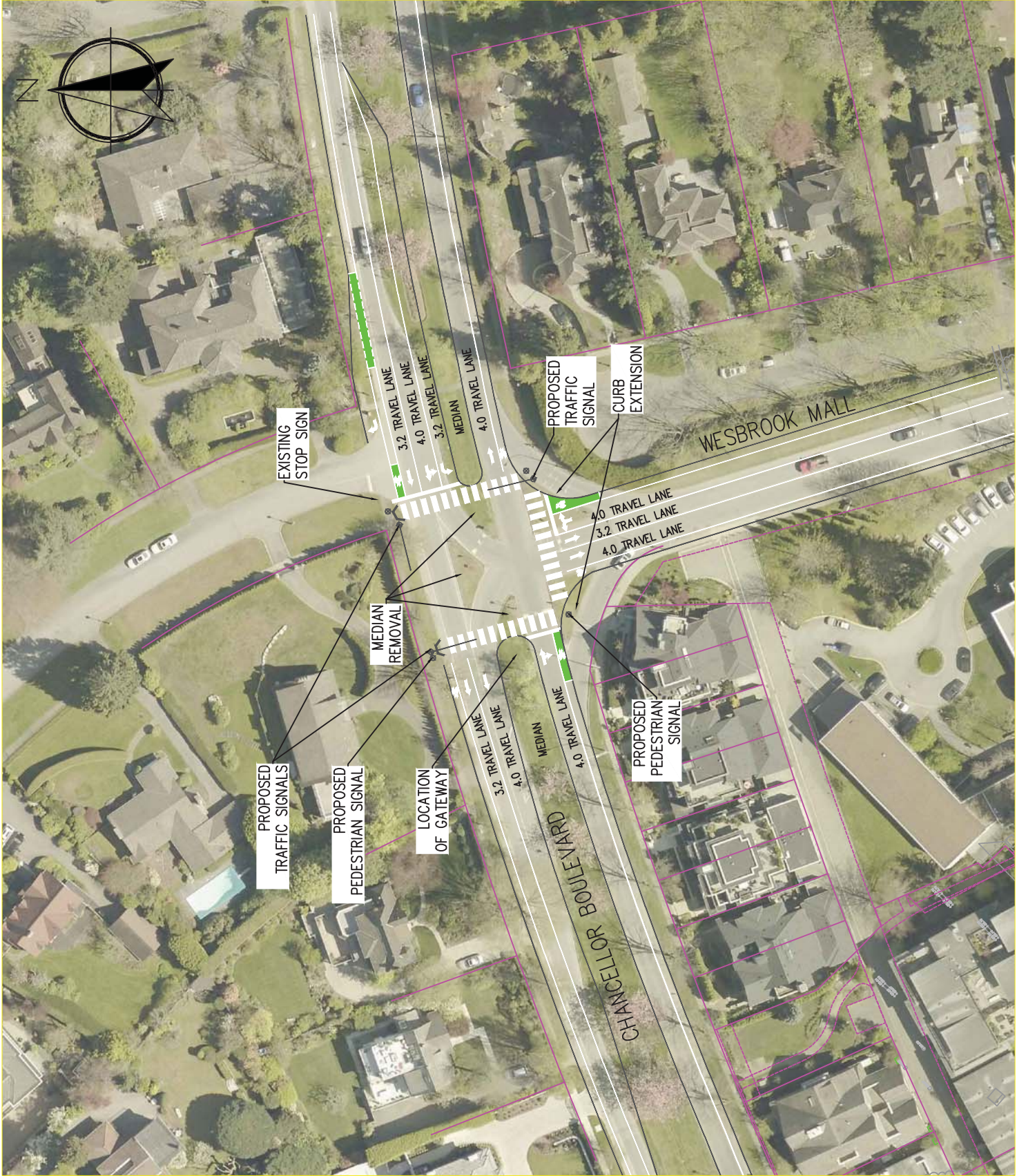
Drawn By: CA

File Name and Address:
TEAM 12

Project Name:
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

Drawing:
DETAILED DESIGN
DRAWING

Client:
UBC SEEDS
Date:
APRIL 7, 2017
Scale:
1:1000
Sheet:
T-01



General Notes

No.	Revisions/Issue	Date

Prepared By
CA

Drawn By
CA

File Name and Address
TEAM 12

Project Name
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

Drawing
GEOMETRIC PLAN
DRAWING

Client UBC SEEDS	Sheet T-02
Date APRIL 7, 2017	Scale 1:1000





General Notes

No.	Revisions/Issue	Date

Prepared By: CA

Drawn By: CA

Team Name and Address:
TEAM 12

Project Name:
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

Drawing:
PAVEMENT MARKINGS
PLAN DRAWING

Client: UBC SEEDS	Sheet: T-03
Date: APRIL 7, 2017	Scale: 1:1000

General Notes

No.	Revisory/Issue	Date

Prepared By
CA

Drawn By
CA

File Name and Address
TEAM 12

Project Name
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

Category
UNDERGROUND UTILITIES
PLAN DRAWING

Client
UBC SEEDS
Date
APRIL 7, 2017
Scale
1:1000
Sheet
T-04



NOTES
1. CONTRACTORS TO CROSS REFERENCE
UNDERGROUND UTILITIES WITH SHEET
XX DURING PAVEMENT REMOVAL TO
ENSURE UNDERGROUND UTILITIES WILL
NOT BE REQUIRED TO BE MOVED

General Notes

No.	Revisions/Issue	Date

Prepared By
CA

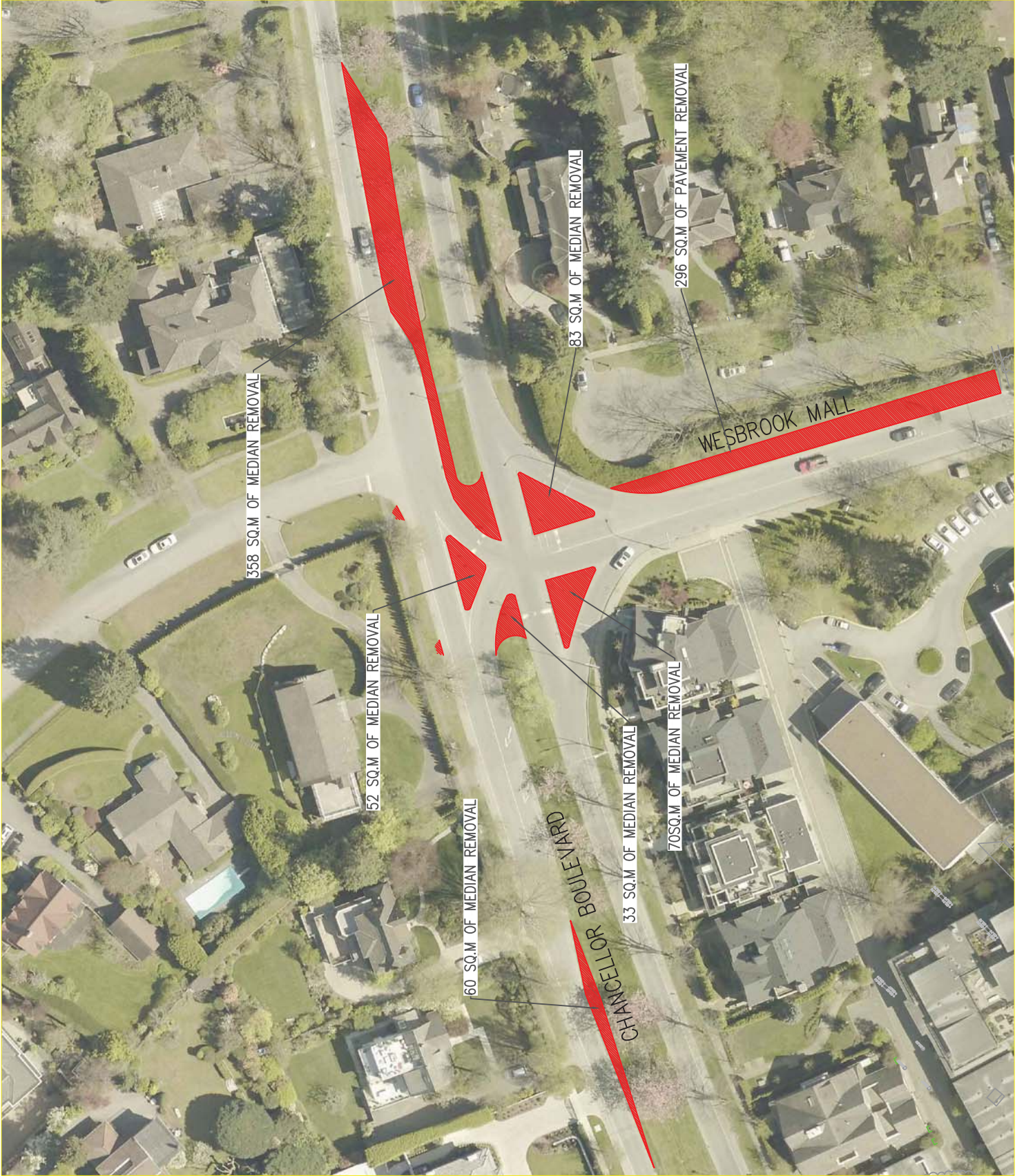
Drawn By
CA

Project Name
TEAM 12

Project Description
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

Drawing
MEDIAN AND PAVEMENT
REMOVAL PLAN DRAWING

Client UBC SEEDS	Sheet T-05
Date APRIL 7, 2017	Scale 1:1000



General Notes

No.	Revisions/Issue	Date

Prepared By
CA

Drawn By
CA

Project Name
TEAM 12

Project Area
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

Drawing
CURB EXTENSION PLAN
DRAWING

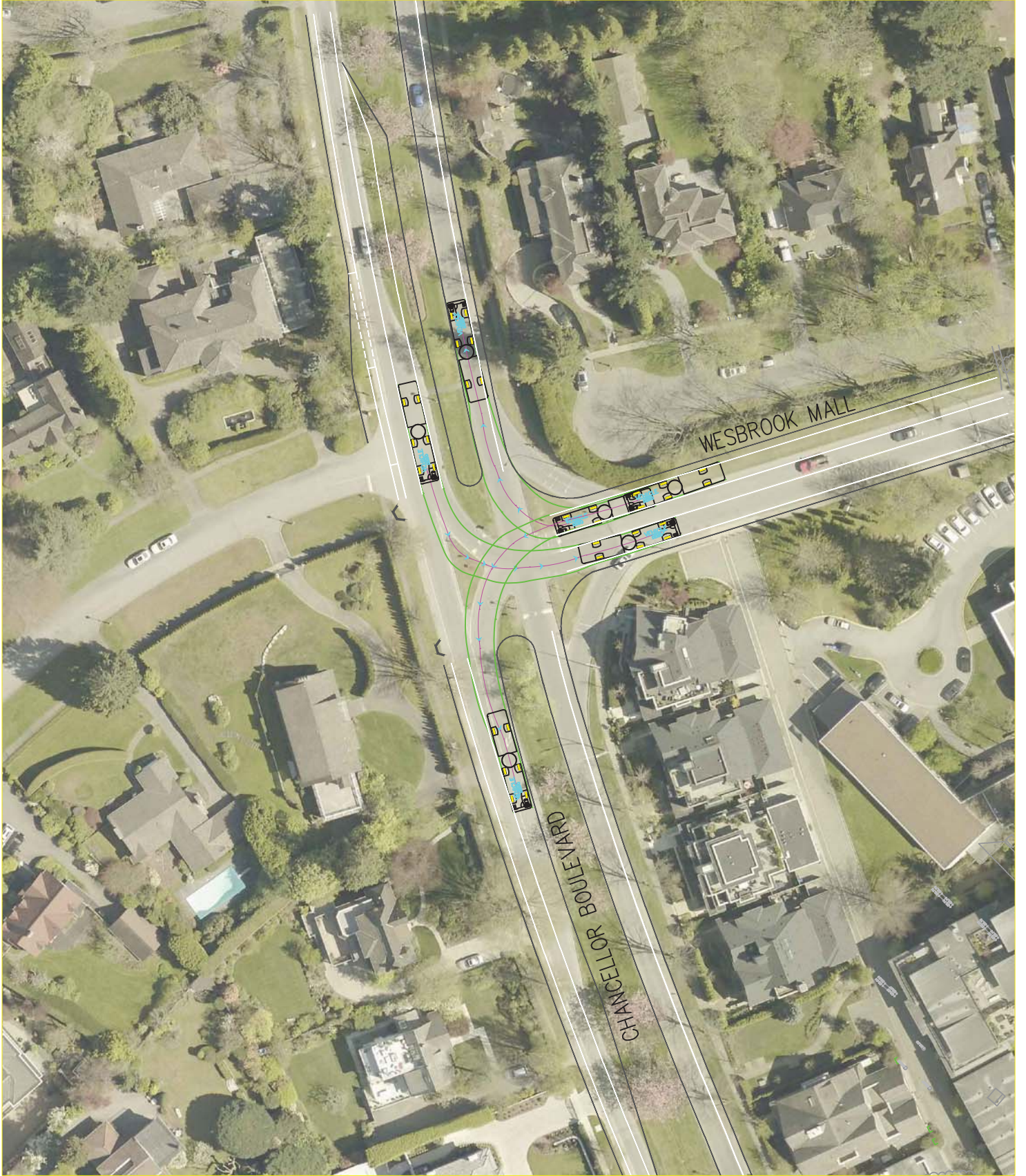
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1:1000

Date
APRIL 7, 2017

Sheet
T-06



NOTES
1. CURB EXTENSIONS TO FOLLOW
DETAILS AS OUTLINED IN SHEET XX



General Notes

No.	Revisions/Issue	Date

Prepared By
CA

Drawn By
CA

Project Name and Address
TEAM 12

Project Name
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

Remarks
AUTOTURN CHECK

Client UBC SEEDS	Sheet T-07
Date APRIL 7, 2017	Scale 1:1000

General Notes

No.	Revisions/Issue	Date

Prepared By
CA

Drawn By
CL

File Name and Address
TEAM 12

Project Name
REDESIGN OF
WEBBROOK MALL
AND
CHANCELLOR BOULEVARD

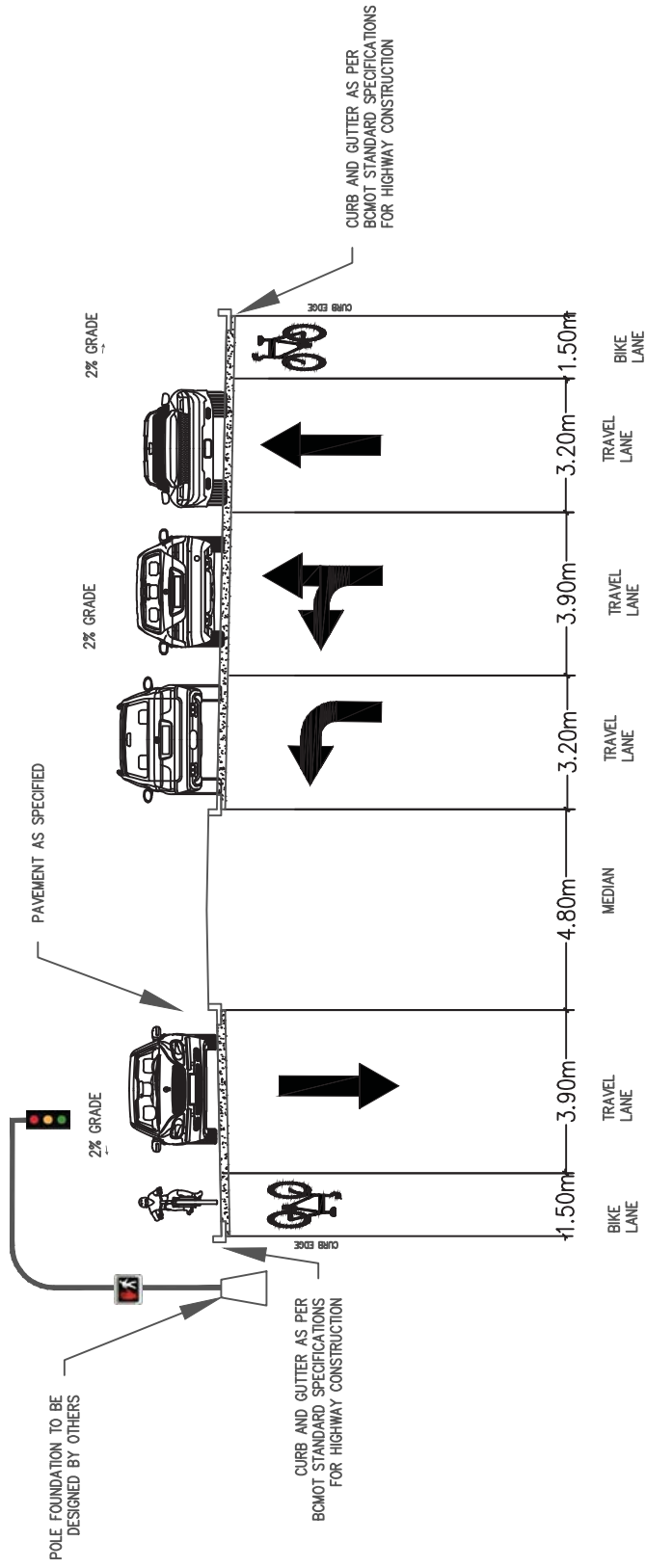
Viewing
EAST SECTION VIEW

Client
UBC SEEDS

Date
APRIL 7, 2017

Sheet
T-08

Scale
NTS



General Notes

No.	Revisions/Issue	Date

Designed by
CA

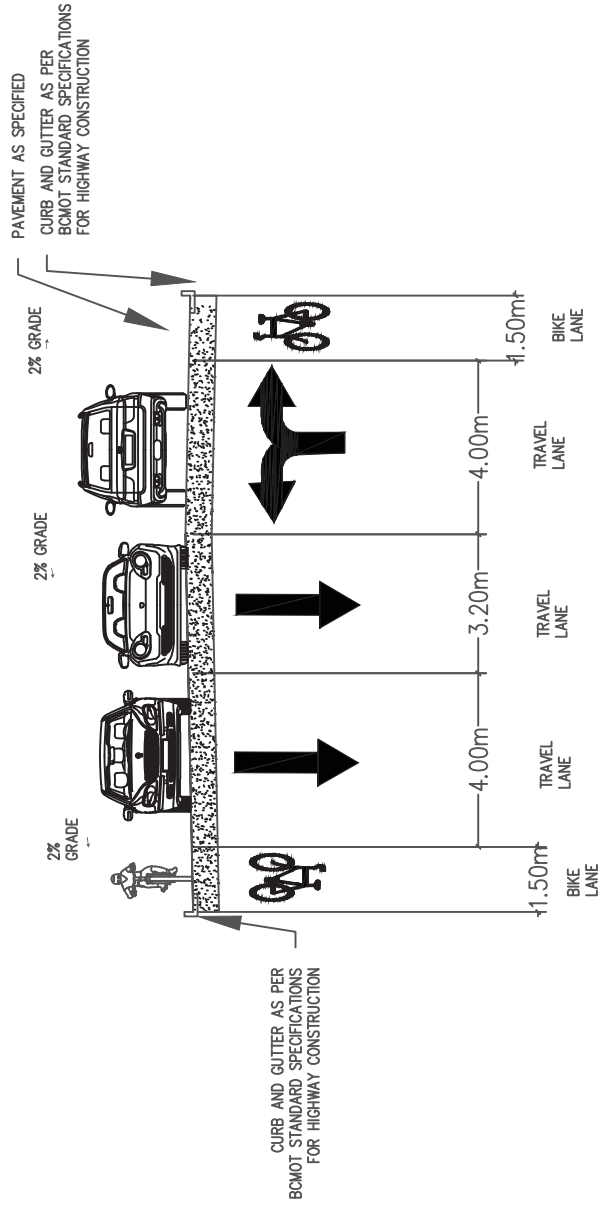
Drawn by
CL

Client Name and Address
TEAM 12

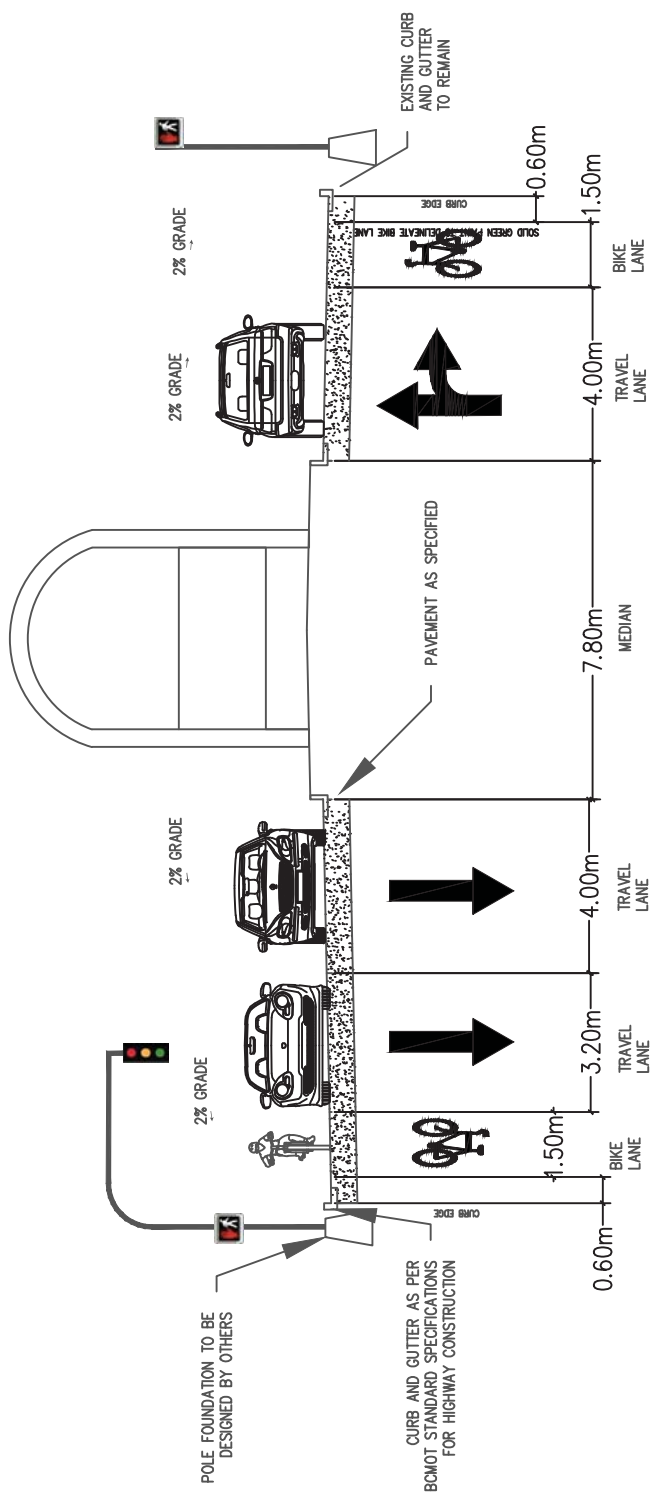
Project Name
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

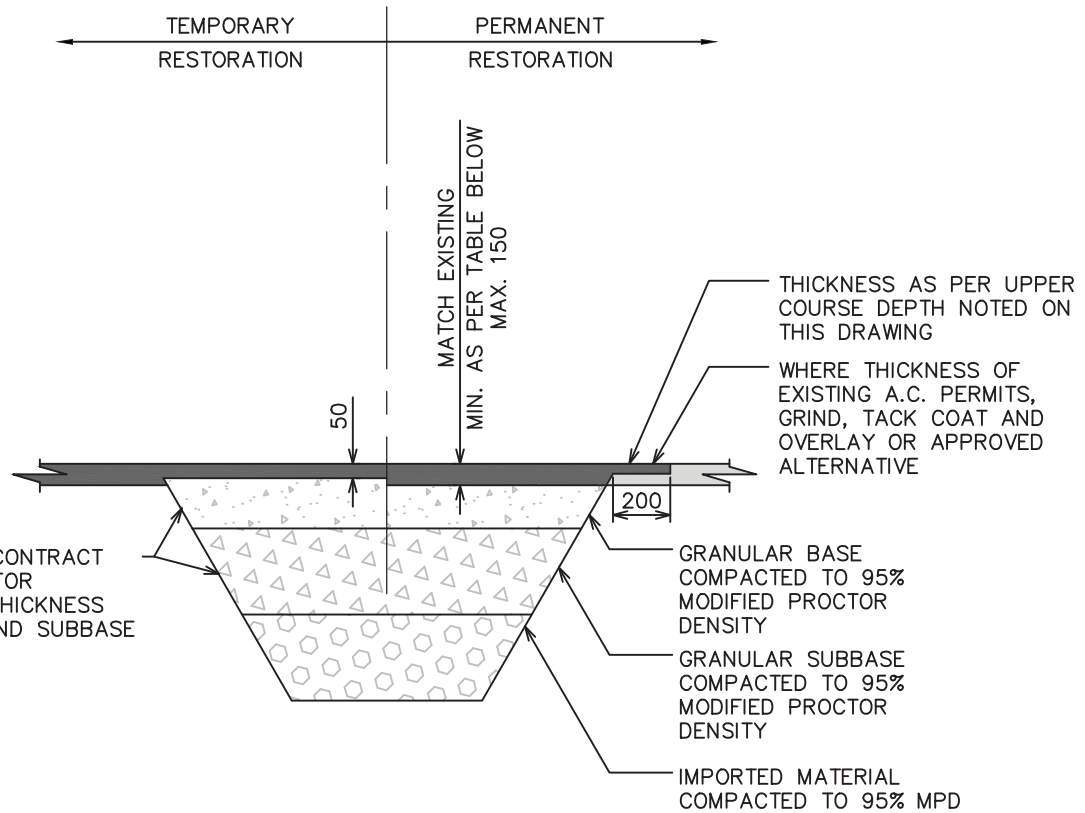
Viewing
SOUTH SECTION VIEW

Client
UBC SEEDS
Date
APRIL 7, 2017
Sheet
T-09
Scale
N.T.S.





General Notes	
No.	Revision/Issue
Date	Date
Designed By CA	Checked By CL
Project Name and Address TEAM 12	
Project Description REDESIGN OF WESBROOK WALL AND CHANCELLOR BOULEVARD	
Viewing WEST SECTION VIEW	
Client UBC SEEDS	Sheet T-10
Date APRIL 7, 2017	Scale N.T.S.

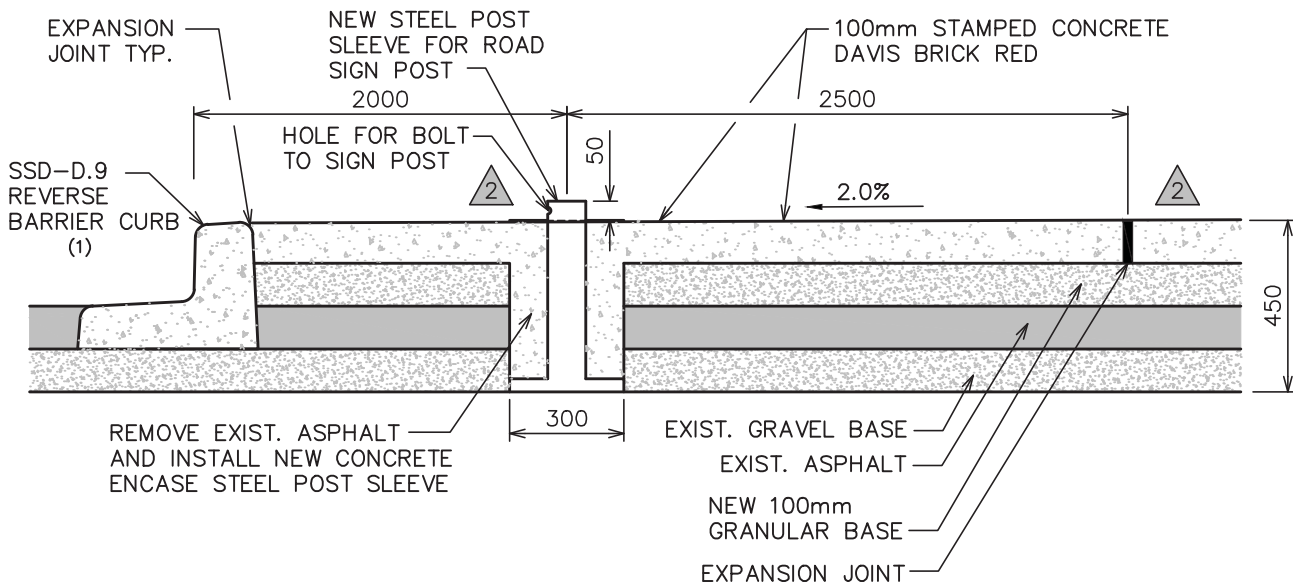
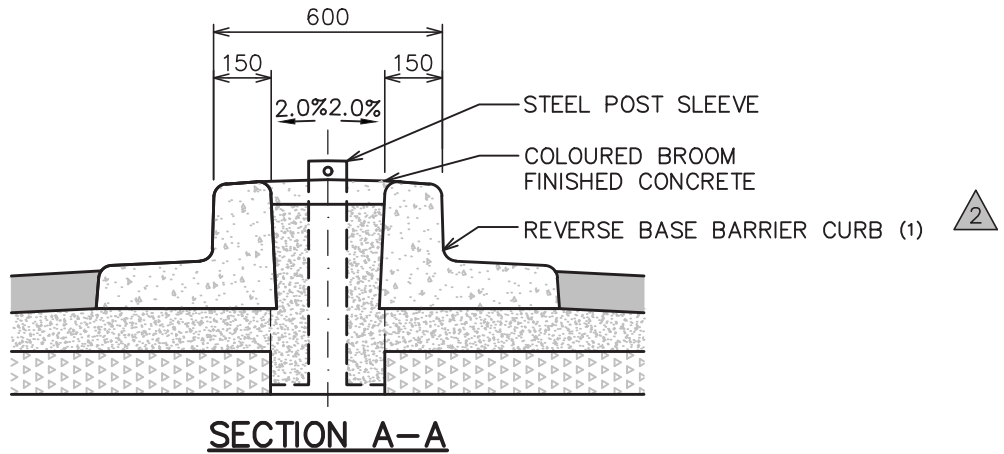






	UPPER COURSE ASPHALT	LOWER COURSE ASPHALT	BASE COURSE	SUB-BASE COURSE
ARTERIALS	50mm SUPER PAVE (12.5mm)	75mm SUPER PAVE (19mm)	100mm 19mm CRUSHED GRANULAR BASE	200mm 75mm CRUSHED GRANULAR SUBBASE
COLLECTORS	40mm UPPER COURSE 1	60mm LOWER COURSE 1	100mm 19mm CRUSHED GRANULAR BASE	200mm 75mm SELECT GRANULAR SUBBASE
LOCAL ROADS ②	40mm UPPER COURSE 2	45mm LOWER COURSE 2	100mm 19mm CRUSHED GRANULAR BASE	200mm 75mm SELECT GRANULAR SUBBASE
NON-RESIDENTIAL LANES & DRIVEWAYS ②	35mm UPPER COURSE 2	40mm LOWER COURSE 2	100mm 19mm CRUSHED GRANULAR BASE	100mm 75mm SELECT GRANULAR SUBBASE
RESIDENTIAL DRIVEWAYS ②	65mm UPPER COURSE 2 (1 LIFTS)		100mm 19mm CRUSHED GRANULAR BASE	100mm 75mm SELECT GRANULAR SUBBASE

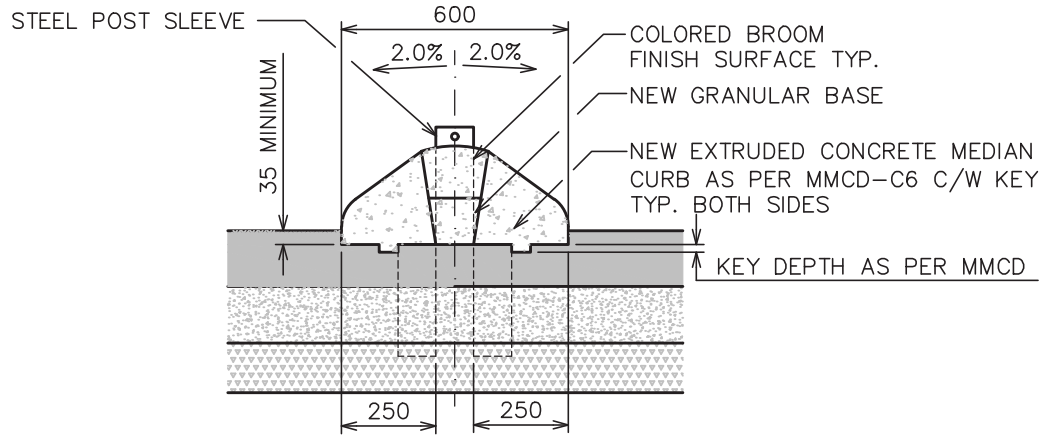
NOTE: 1. REFER TO CONTRACT DOCUMENT SECTIONS 31 23 01, 32 12 16 AND 32 12 17 FOR SPECIFICATIONS.
2. BUTT JOINT TO HAVE 600mm MINIMUM OVERLAP IN LATERAL DIRECTION.

3			All Dimensions Shown In Millimetres, Unless Otherwise Noted	
②	JULY 2016	SCOTT NEUMAN		
1	JANUARY 2016	SCOTT NEUMAN	Title SURFACE ASPHALT REINSTATEMENT AND STANDARD ROAD STRUCTURE SECTIONS	
	Revision Date	Approved		
		SUPPLEMENTARY STANDARD DRAWINGS	Approved By :  G.M. Engineering	DRAWING NUMBER SSD-G.5
			JANUARY 2016	

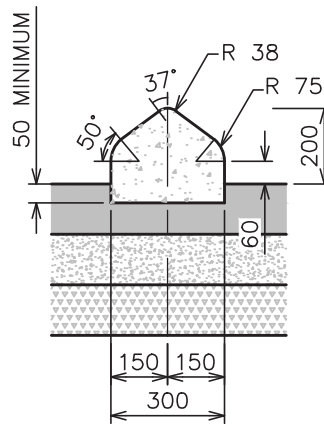


2 NOTES: (1) CURB AND GUTTER TO BE BULL NOSE END TO BE NARROW BASE BARRIER CURB AS PER MMCD DWG C4

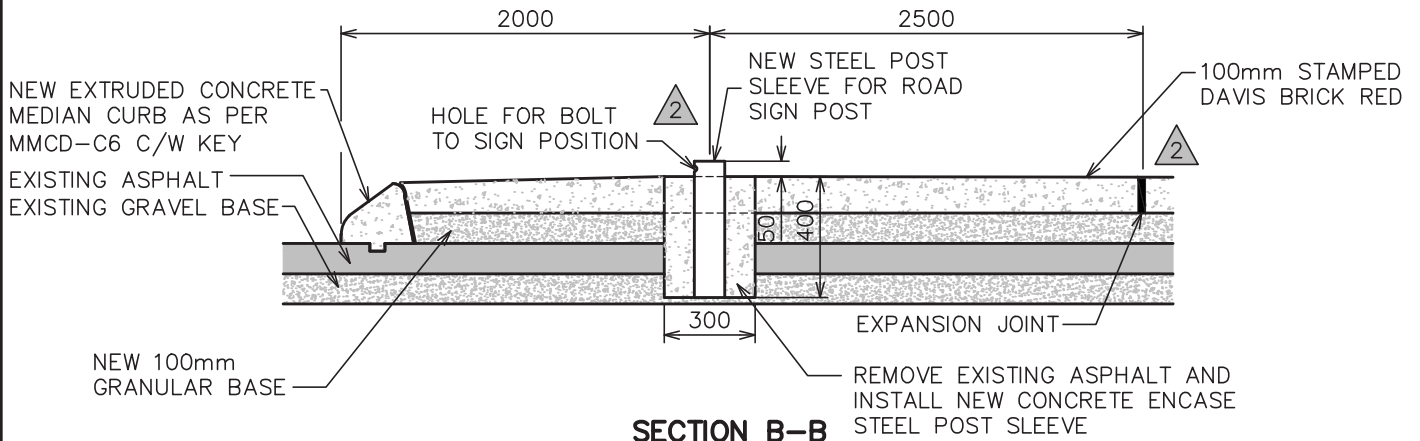
3			All Dimensions Shown In Millimetres, Unless Otherwise Noted
2	JULY 2016	JAIME BOAN	
1	JANUARY 2016	JAIME BOAN	Title RAISED MEDIAN, BULL NOSE END BARRIER CURB DETAIL
	Revision Date	Approved	
 SUPPLEMENTARY STANDARD DRAWINGS			Approved By :  JANUARY 2016 G.M. Engineering
			DRAWING NUMBER SSD-R.15.2



**SECTION A-A
MEDIAN AT LEFT TURN BAY
TYP. SECTION**



**EXTRUDED CONCRETE CURB CENTER
DIVIDER NARROW SECTION DETAIL**



SECTION B-B

3		
2	JULY 2016	JAIME BOAN
1	JANUARY 2016	JAIME BOAN
	Revision Date	Approved

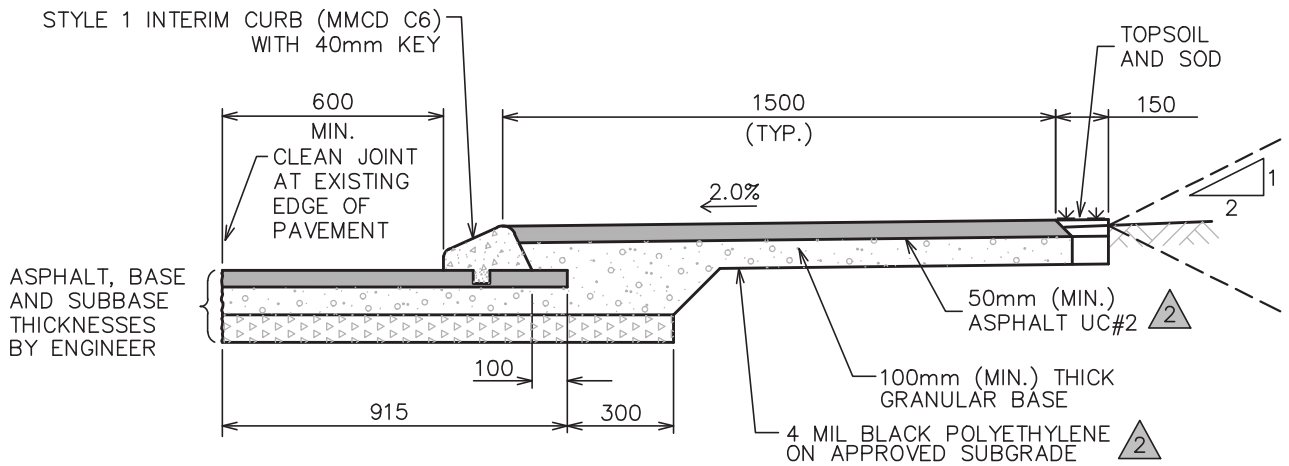
All Dimensions Shown In Millimetres,
Unless Otherwise Noted

Title **RAISED MEDIAN, BULL NOSE END
EXTRUDED CURB DETAIL**

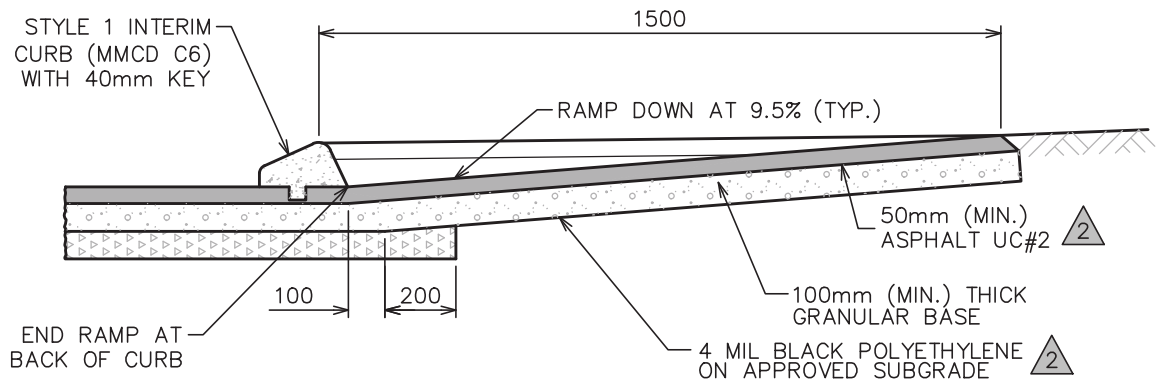


Approved By : *J. Smith*
JANUARY 2016 G.M. Engineering



DRAWING NUMBER
SSD-R.15.3



ELEVATED ASPHALT SIDEWALK

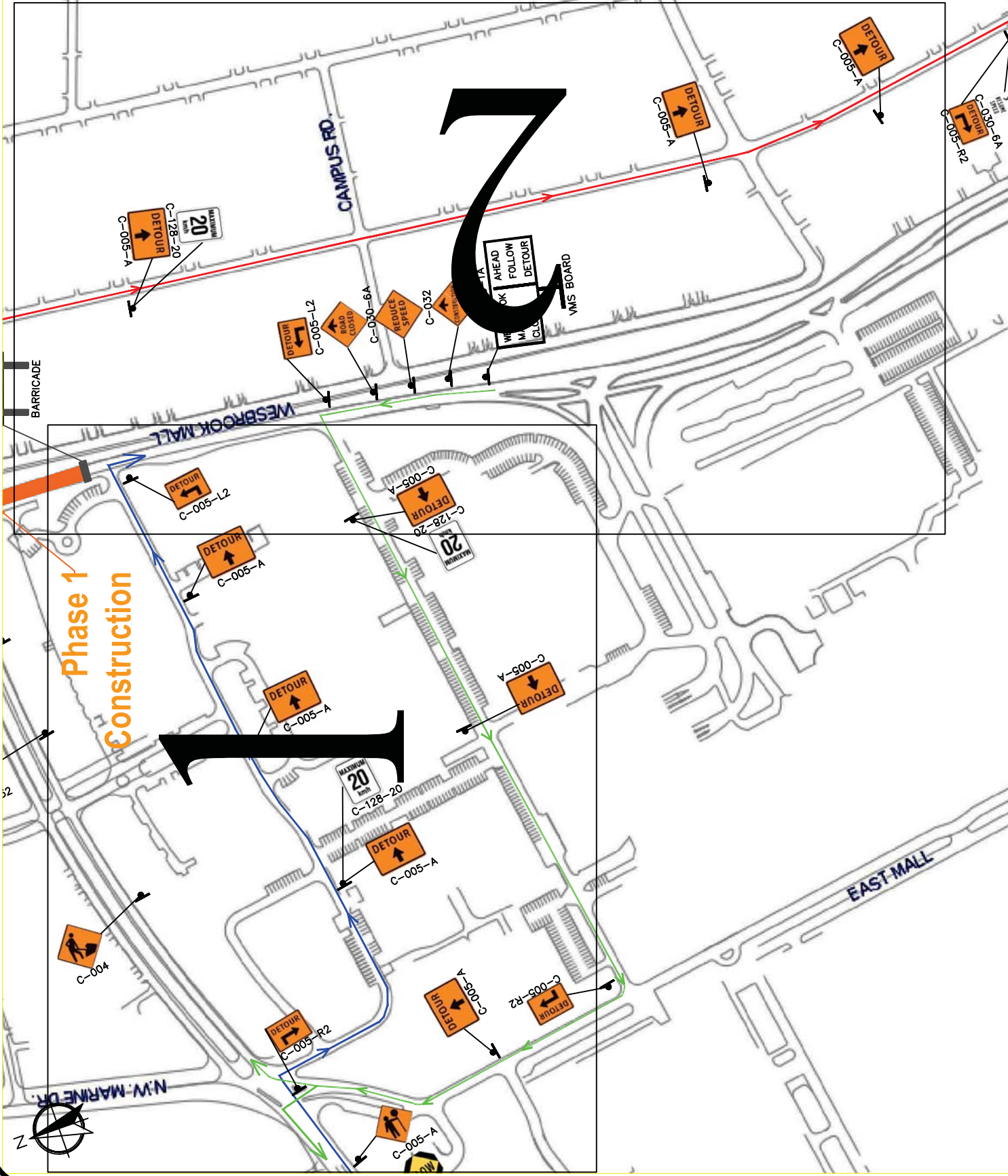


ASPHALT SIDEWALK AT WHEELCHAIR RAMP

3			All Dimensions Shown In Millimetres, Unless Otherwise Noted
2	JULY 2016	JAIME BOAN	
1	JANUARY 2016	JAIME BOAN	
	Revision Date	Approved	Title SIDEWALKS, INTERIM ASPHALT
 SUPPLEMENTARY STANDARD DRAWINGS			Approved By :  JANUARY 2016 G.M. Engineering
			DRAWING NUMBER SSD-R.27

General Notes	No.	Revisions/Issues	Date

Prepared By	YL
Drawn By	YL
File Name and Address	TEAM 12
Project Name	REDESIGN OF WEBBROOK MALL AND CHANCELLOR BOULEVARD
	TRAFFIC ACCOMMODATION PLAN-PHASE 1
Client	UBC SEEDS
Date	APRIL 7, 2017
Scale	1:1000
Sheet	T-12



**Phase 1
Construction**

General Notes

No.	Revisory/Issue	Date

Prepared by: YL

Checked by: YL

Project Name: TEAM 12

Project Area: REDESIGN OF WESBROOK MALL AND CHANCELLOR BOULEVARD

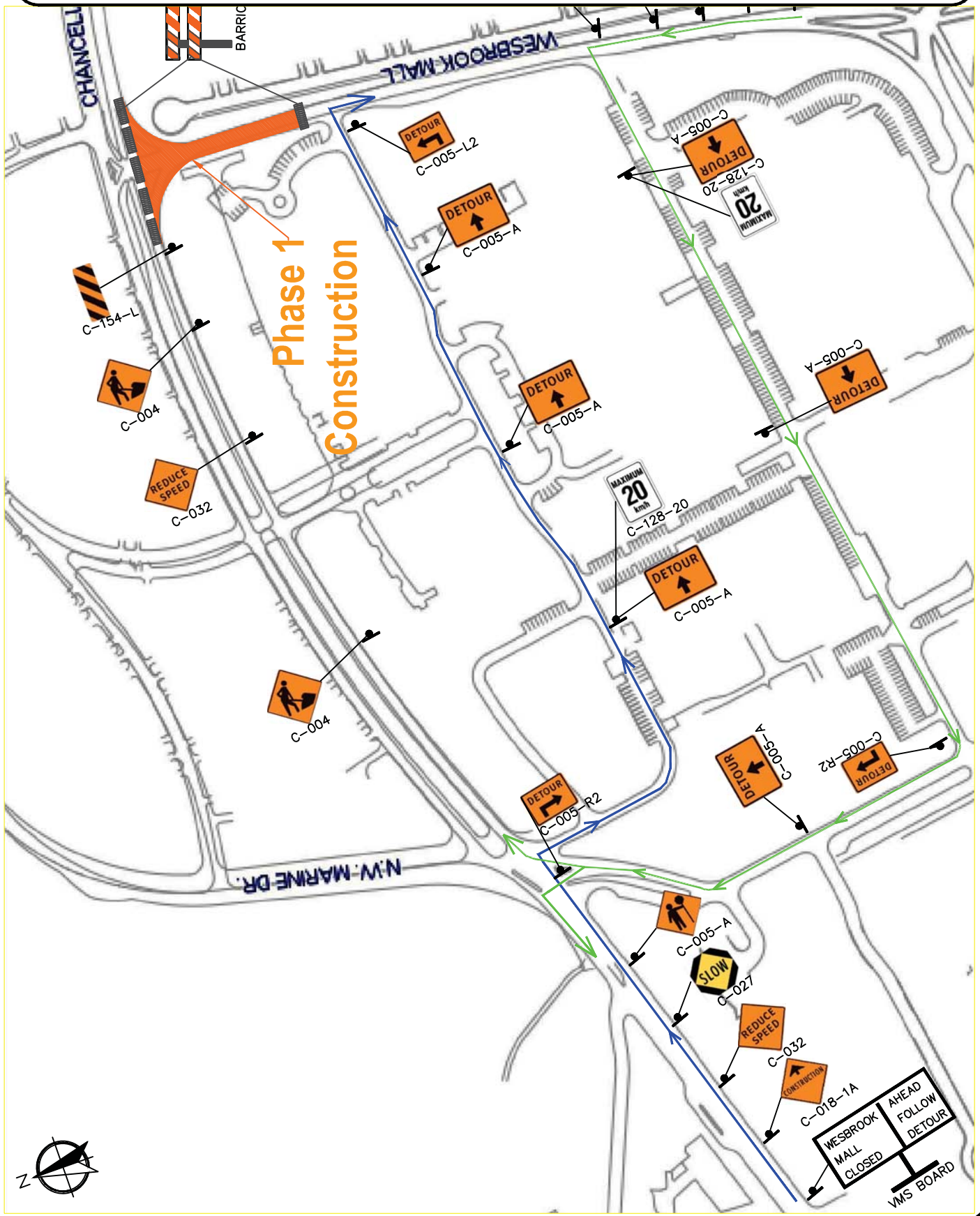
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Client: UBC SEEDS

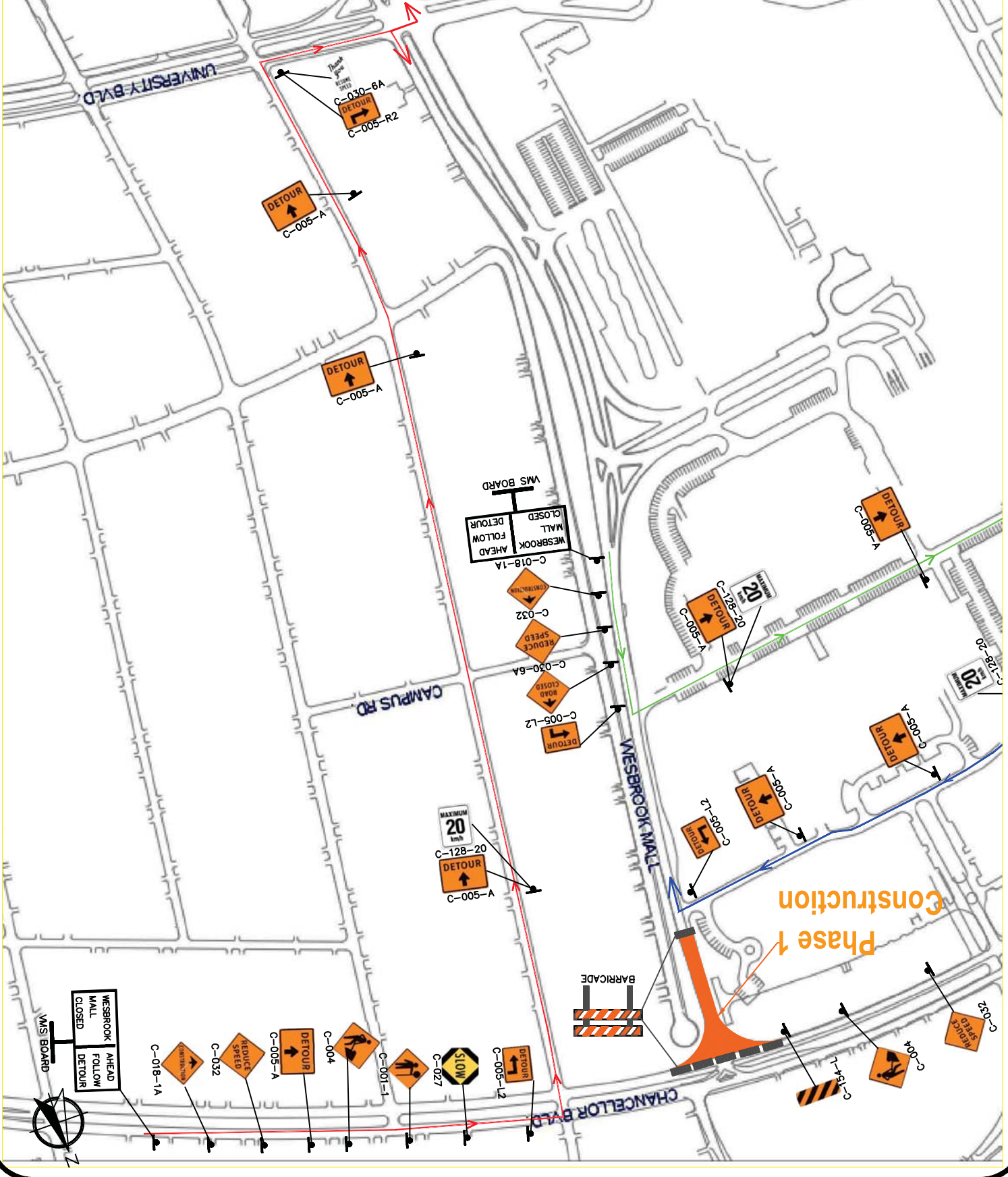
Date: APRIL 7, 2017

Sheet: T-13

Scale: 1:500



General Notes		Revisory/Issue	Date
Approved By	YL		
Drawn By	YL		
File Name and Address	TEAM 12		
Project Name	REDESIGN OF WESBROOK MALL AND CHANCELLOR BOULEVARD		
Category	TRAFFIC ACCOMMODATION PLAN—PHASE 1 SHEET 2		
Client	UBC SEEDS	Sheet	T-14
	APRIL 7, 2017		
		Scale	1:500



General Notes

Revised/Issue

Date

Prepared by

Y.L.

Checked by

Y.L.

File Name and Address

TEAM 12

Project Name
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

Planning
TRAFFIC ACCOMMODATION
PLAN-PHASE 2

Scale
1:500
Date
APRIL 7, 2017
Sheet
T-15



General Notes

No.	Revisions/Issues	Date

Prepared by: YL

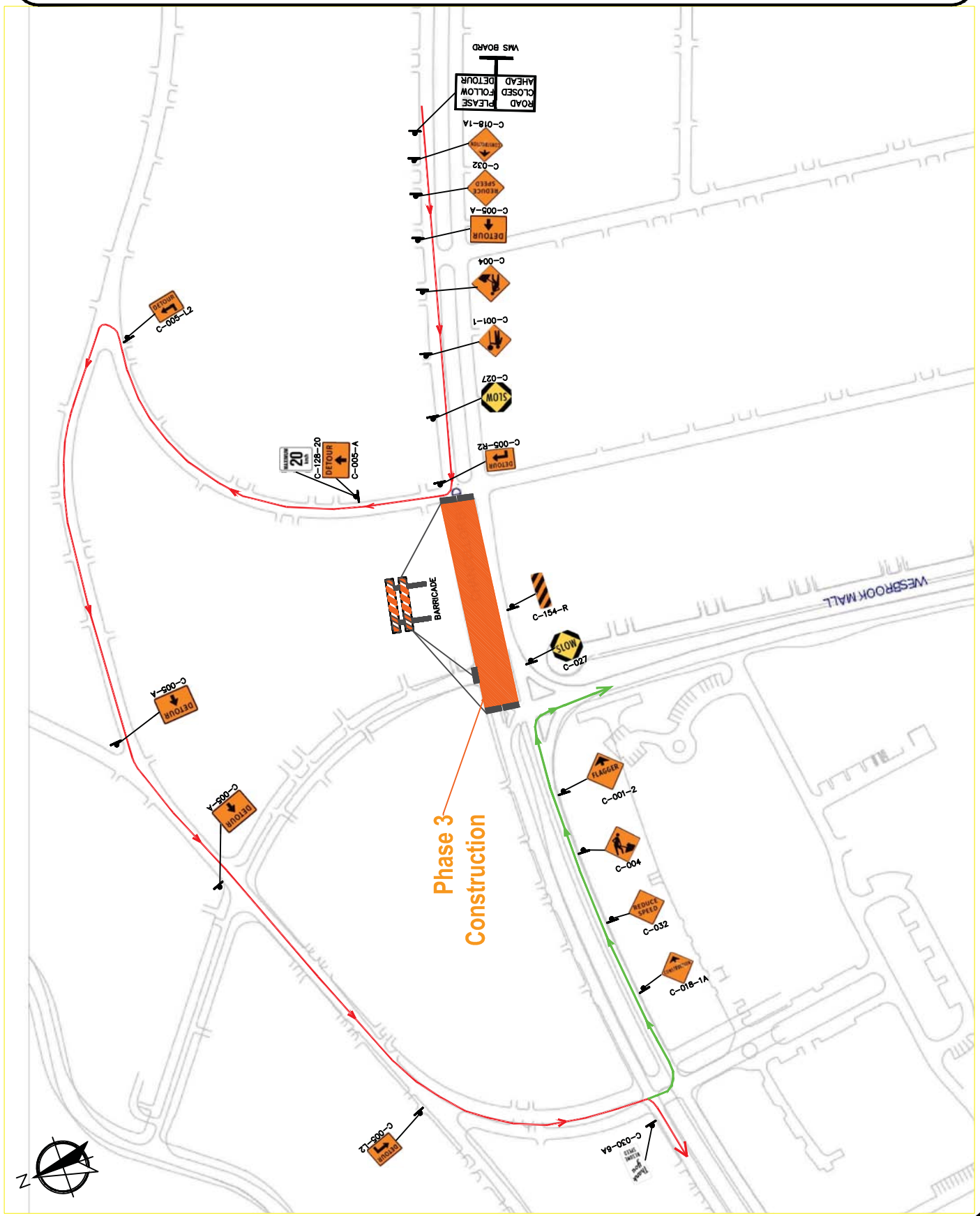
Drawn by: YL

File Name and Address: TEAM 12

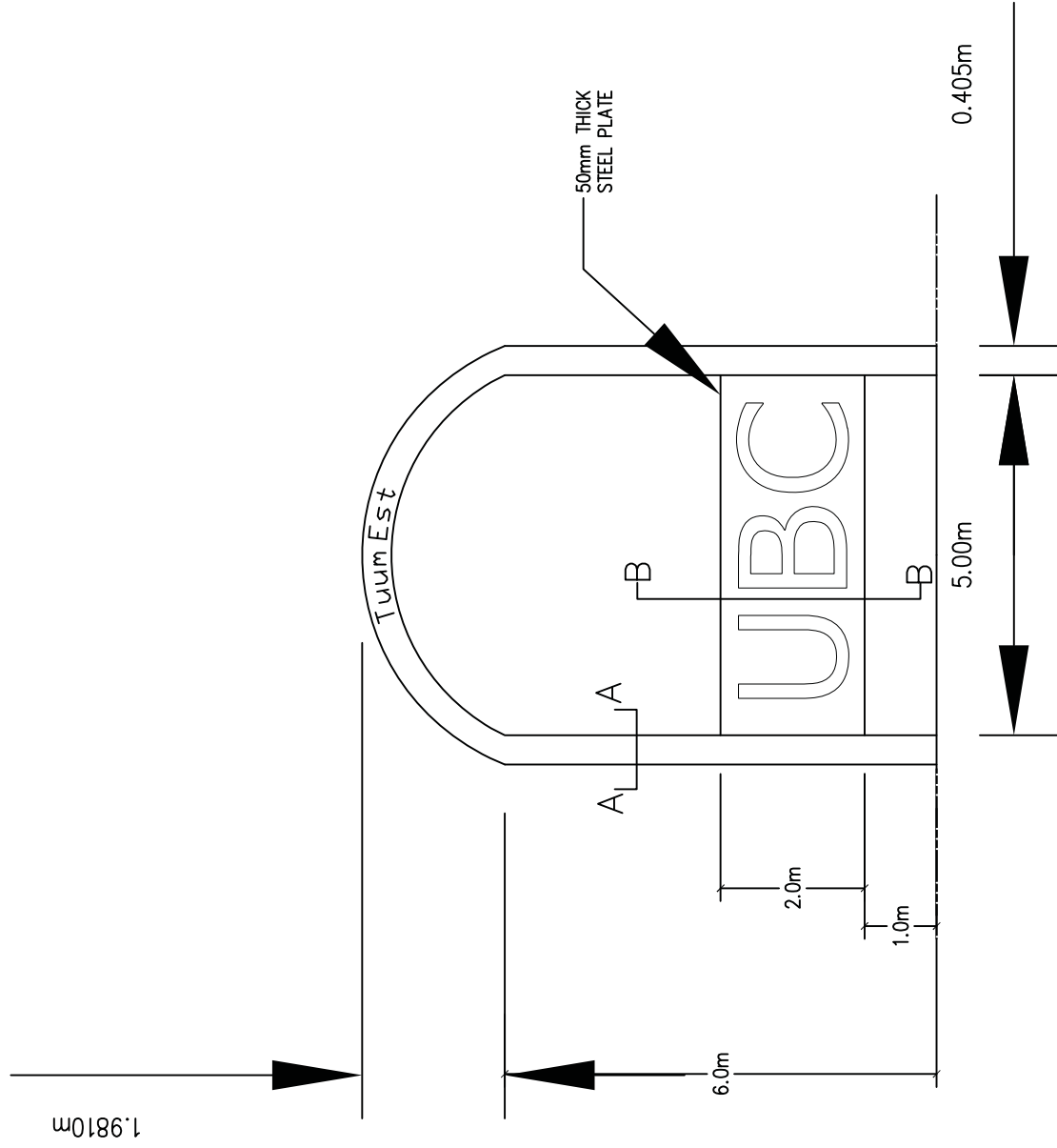
Project Name: REDESIGN OF WESBROOK MALL AND CHANCELLOR BOULEVARD

Package: TRAFFIC ACCOMMODATION PLAN—PHASE 3

Client: UBC SEEDS
 Date: APRIL 7, 2017
 Sheet: T-16
 Scale: 1:500



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No.															
Revision/Issue															
Date															
<table border="1"> <tr><td>Designed By</td><td>MT</td></tr> </table>	Designed By	MT	<table border="1"> <tr><td>Drawn By</td><td>MT</td></tr> </table>	Drawn By	MT	<table border="1"> <tr><td>Client Name and Address</td><td>TEAM 12</td></tr> </table>	Client Name and Address	TEAM 12	<table border="1"> <tr><td>Project Name</td><td>REDESIGN OF WESBROOK MALL AND CHANCELLOR BOULEVARD</td></tr> </table>	Project Name	REDESIGN OF WESBROOK MALL AND CHANCELLOR BOULEVARD				
Designed By	MT														
Drawn By	MT														
Client Name and Address	TEAM 12														
Project Name	REDESIGN OF WESBROOK MALL AND CHANCELLOR BOULEVARD														
<table border="1"> <tr><td>Contractor</td><td>STRUCTURAL ARCH</td></tr> </table>			Contractor	STRUCTURAL ARCH	<table border="1"> <tr><td>Client</td><td>UBC SEEDS</td></tr> <tr><td>Date</td><td>APRIL 7, 2017</td></tr> <tr><td>Scale</td><td>1:100</td></tr> <tr><td>Sheet</td><td>S01</td></tr> </table>	Client	UBC SEEDS	Date	APRIL 7, 2017	Scale	1:100	Sheet	S01		
Contractor	STRUCTURAL ARCH														
Client	UBC SEEDS														
Date	APRIL 7, 2017														
Scale	1:100														
Sheet	S01														



General Notes

No.	Revision/Issue	Date

Designed By
MT

Checked By
MT

Project Name
TEAM 12

Project Location
REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

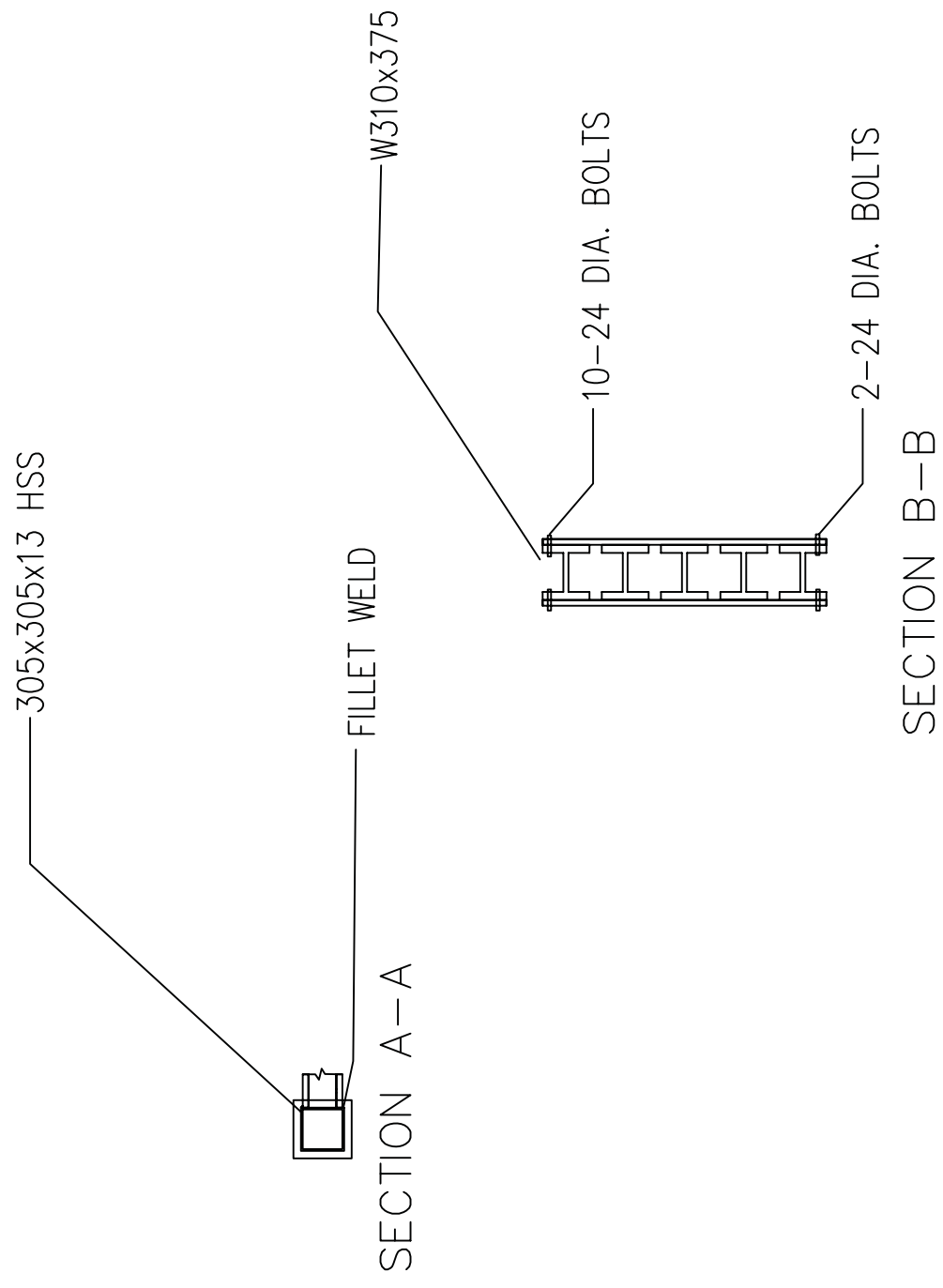
Category
STRUCTURAL DETAILS

Client
UBC SEEDS

Date
APRIL 7, 2017

Scale
1:50

Sheet
S02



General Notes

No.	Revision/Issue	Date

Designed By

MT

Drawn By

MT

Client Name and Address

TEAM 12

Project Name

REDESIGN OF
WESBROOK MALL
AND
CHANCELLOR BOULEVARD

Drawing

FOOTING DETAIL

Client

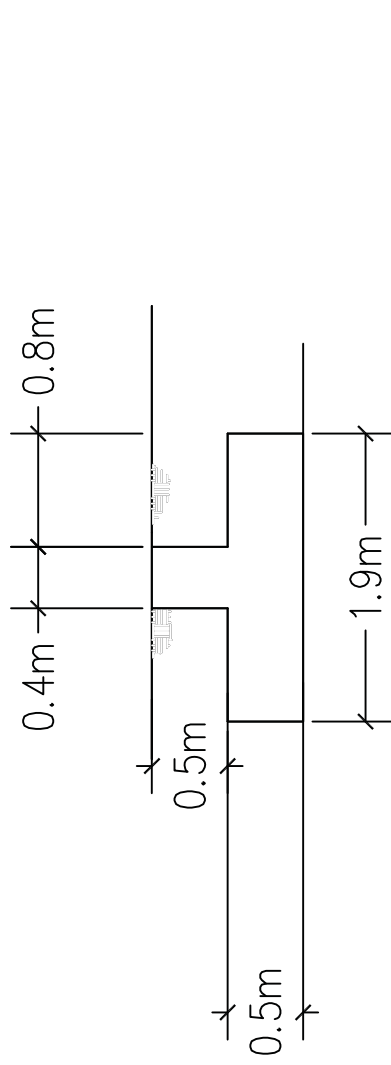
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APRIL 7, 2017

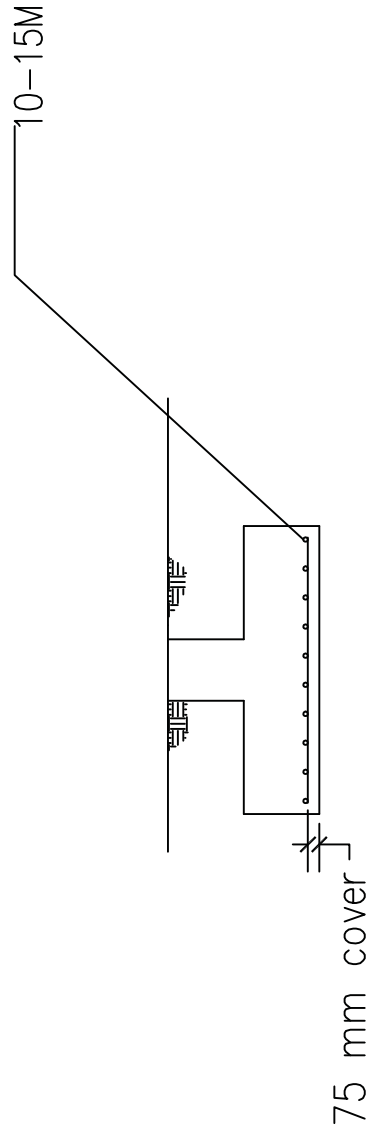
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1:50

Sheet

S03



FOOTING



TYPICAL FOOTING DETAIL

APPENDIX B : COST ESTIMATE BREAKDOWN

Team Member	Mandy Tam	Cliff Chun	Evan Yan	Celine Au	York Liao	Chi Yan Leung	Dr. Armin Bebamzadeh
Qualification	EIT	EIT	EIT	EIT	EIT	EIT	Supervisory Engineer
Professional Service Hourly Rate	\$ 121.00	\$ 121.00	\$ 121.00	\$ 121.00	\$ 121.00	\$ 121.00	\$ 195.00
Detailed Design Phase							
Team Meetings (hr)	3	3	3	3	3	3	0
Site Visits (hr)	1	1	1	1	1	1	0
Review Meetings (hr)	1	1	1	1	1	1	1
Research (hr)	2	2	2	2	2	2	0
Field Survey (hr)	1.5	1.5	1.5	1.5	1.5	1.5	0
Detailed Design: Geotechnical Analysis (hr)	1	1	1	1	1	1	0
Detailed Design: Intersection (hr)	1.5	1.5	1.5	3	1.5	1.5	0
Detailed Design: Gateway (hr)	5	3	1	1	1	3	0
Drafting and Modeling (hr)	2.5	1	1	1	1	1	0
Schedule (hr)	1	1	1	1	5	1	0
Cost (hr)	1	1	6.5	1	1	1	0
Permit Application Preparation (hr)	2	2	2	2	2	2	0
Client Consultation for Detailed Design (hr)	1.5	1.5	1.5	1.5	1.5	1.5	0
Summary Design Report (hr)	2	2	2	3.5	2	3	0
Presentation Preparation (hr)	2	2	2	4	2	2	0
Detailed Design Presentation (hr)	1	1	1	1	1	1	1
Sub-total Cost	\$ 3,509.00	\$ 3,085.50	\$ 3,509.00	\$ 3,448.50	\$ 3,327.50	\$ 3,206.50	\$ 390.00
Minor Disbursements (5%)	\$						1,023.80
Total Cost for Detailed Design	\$						21,499.80
Procurement Phase							
Team Meetings (hr)	2	2	2	2	2	2	0
Review Meetings (hr)	0.5	0.5	0.5	0.5	0.5	0.5	0.5
IFT for Contractor	1.5	1.5	1.5	1.5	1.5	1.5	0
Bid Package Preparation for Tender	4	4	4	4	4	4	0
Contractor Submission Period	2	2	2	2	2	2	0
Bid Selection Process	3	3	3	3	3	3	0
Client Consultation for Selected Bid (hr)	1	1	1	1	1	1	0
Bid Award to Contractor	1	1	1	1	1	1	0
Sub-total Cost	\$ 1,815.00	\$ 1,815.00	\$ 1,815.00	\$ 1,815.00	\$ 1,815.00	\$ 1,815.00	\$ 97.50
Minor Disbursements (5%)	\$						549.38
Total Cost for Procurement Phase	\$						11,536.88
Summary							
Total Cost for Detailed Design	\$						21,499.80
Total Cost for Procurement Phase	\$						11,536.88
Total Cost of Consultation	\$						33,036.68

Detailed Construction Cost Breakdown

Mobilization	\$ 8,700.0
Communication	\$ 1,050.0
Toilets	\$ 600.0
Office Supplies	\$ 3,000.0
Transportation	\$ 3,000.0
Trailer	\$ 1,050.0
Site Preparation	\$ 12,400.0
Temporary Service	\$ 4,100.0
Site tools, Equipment	\$ 2,000.0
Safety Provision	\$ 1,500.0
Dampproofing	\$ 300.0
Firestopping	\$ 500.0

Survey	\$ 4,000.0
Phase 1 - Wesbrook Mall	\$ 488,779.0
Detour Setup	\$ 6,500.0
Earthwork (Island Removal)	
Excavation, Trenching	\$ 2,154.0
Backfill, Compaction	\$ 4,274.0
Earthwork (Road)	
Excavation, Trenching	\$ 3,431.1
Backfill, Compaction	\$ 6,808.0
Utility Trenching	\$ 512.0
Traffic Signals & Pedestrian Flashers Installation	\$ 459,000.0
Sidewalk Construction	\$ 500.0
Curb & Gutter Extension	\$ 5,600.0
Phase 2 - West Leg Chancellor Blvd Widening	\$ 26,683.4
Detour Setup	\$ 12,500.0
Earthwork (Road)	
Excavation, Trenching	\$ 1,102.8
Backfill, Compaction	\$ 2,188.1
Utility Trenching	\$ 164.5
Median Construction + Curb & Gutter Work	\$ 4,480.0
Structural Gateway Construction	\$ 6,248.0
Earthwork	\$ 763.0
Gateway Footing Construction	\$ 1,856.0
Gateway Column Construction	\$ 1,538.0
Gateway Concrete Arc Construction	\$ 1,841.0
Gateway Finishing	\$ 250.0
Phase 3 - East Leg Chancellor Blvd Widening	\$ 35,969.1
Detour Setup	\$ 7,300.0
Earthwork (Road)	
Tree Removal	\$ 6,000.0
Excavation, Trenching	\$ 4,711.2
Backfill, Compaction	\$ 9,347.9
Median Construction + Curb & Gutter Work	\$ 8,610.0
Re-Surfacing	\$ 46,867.6
Asphalt	\$ 24,000.0
Final Asphalt	\$ 5,387.6
Lane Paint	\$ 12,800.0
Road Markings	\$ 2,880.0
Bike Lane Signs	\$ 1,800.0
Demobilization	\$ 5,000.0
Waste Management	\$ 5,000.0

Completion Inspection	\$ 1,000.0
Handover	\$ 380.0
General Contractor Miscellaneous	\$ 182,255.0
Insurance and Bonds	\$ 2,500.0
Material Testing	\$ 3,500.0
Permits	
BC Minister of Transportation	\$ 1,008.0
UBC	\$ 5,772.0
Courier	\$ 300.0
Staff	\$ 169,175.0
Subtotal	\$ 808,034.0
6% Contingency	\$ 48,482.0
Total	\$ 856,516.1
5% GST	\$ 42,825.8
Final	\$ 899,341.9

Unit Rates

Labor	\$/hr
<i>Superintendent</i>	58.21
<i>Foreman</i>	49.17
<i>Senior Project Manager</i>	70
<i>Project Manager</i>	65
<i>Project Coordinator</i>	35
<i>Safety Officer</i>	50
<i>Site Coordinator</i>	35
<i>Flagger</i>	25

Materials	\$	unit
<i>Pit Run</i>	25	\$/m3
<i>Drain Rock</i>	35	\$/m3
<i>Asphalt</i>	24	\$/m2
<i>Final Asphalt</i>	5	\$/m2
<i>Tip Fee</i>	20	\$/load
<i>Paint Bike Lane</i>	80	\$/m
<i>Curb</i>	70	\$/m
<i>Bike Marking</i>	180	\$/ea
<i>Bike Lane Signs</i>	300	\$/ea
<i>Traffic Signal</i>	150000	\$/ea
<i>Traffic Signal Installation</i>	3000	\$/ea
<i>Trailers</i>	350	\$/month

Equipment	(Labour) \$/hr	(Equipment) \$/hr
------------------	-----------------------	--------------------------

<i>Median Excavator</i>	39.2	140
<i>Truck</i>	/	100
<i>bobcat</i>	40.48	65
<i>Roller</i>	41.31	65

Sample Calculations

Sample calculations for staff cost. Other general contractor's costs are estimated similarly

Description	Qty	Unit	Labour			Material	Equipment		Subcontract								
			man hr	total mh	Rate	Rate	Qty	Rate	Qty	Rate	Labour	Material	Equip.	Sub.	Total		
Staff																	
project start-up																	
Project Manager	1	wk	32	32	65							2,080	-	-	-	-	2,080
Project Co-Ordinator / Student	1	wk	16	16	35							560	-	-	-	-	560
project duration																	
Project Manager	9	wk	32	288	65							18,720	-	-	-	-	18,720
Project Co-Ordinator / Student	9	wk	8	72	35							2,520	-	-	-	-	2,520
Safety Staff																	
Safety Officer - office work	9	wk	40	360	50							18,000	-	-	-	-	18,000
Safety Audits & Training	9	wk	2	18													
Site Staff																	
Superintendent	10	wk	32	320	58.21							18,627	-	-	-	-	18,627
Foreman	10	wk	40	400	49.17							19,668	-	-	-	-	19,668
Site Coordinator x5	50	wk	40	2,000	35							70,000	-	-	-	-	70,000
Flagger x2	19	wk	40	760	25							19,000	-	-	-	-	19,000
Subtotal	169,175																

Sample calculations for excavation cost

Description	Qty	Unit	Labour			Material	Equipment		Subcontract								
			man hr	total mh	Rate	Rate	Qty	Rate	Qty	Rate	Labour	Material	Equip.	Sub.	Total		
Backfill																	
sub-base																	
Volume:	216	m3															
Waste & Compaction:	115%																
Import Material:	248	m3				25						-	6,196	-	-	-	6,196
Productivity	25	m3/hr															
MEX Placing	9.91	hrs	1	10	39.2			1	140			388	-	1,388	-	-	1,776
Bobcat Spread	9.91	hrs	1	10	40.48			1	65			401	-	644	-	-	1,045
Roller Compact	9.91	hrs	1	10	41.31			1	65			409	-	644	-	-	1,053
subtotal:	10,070																
Island =	1,903																
Phase 1 =	3,031																
Phase 2 =	974																
Phase 3 =	4,162																
\$/m3 (neat volume):	46.7																

Sample calculations for costs of backfilling sub-base. Costs for base and asphalt are done in a similar way.

Description	Qty	Unit	Labour			Material	Equipment		Subcontract								
			man hr	total mh	Rate	Rate	Qty	Rate	Qty	Rate	Labour	Material	Equip.	Sub.	Total		
Excavation																	
neat volume =	592,6305	m3															
bulk factor =	120%																
bulk volume =	711	m3															
MEX production rate =	35	m3 / hr															
MEX hours =	20	hrs	1	20	39.2			1	140			796	-	2,845	-	-	3,641
Trucking to dump site																	
truck volume =	11.00	m3															
Trucking to dump site	711	m3															
calculated loads =	65	ea						1	20			-	-	1,293	-	-	1,293
cycle time off site =	1	hrs															
calculated truck hours =	64.65	hrs						1	100			-	-	6,465	-	-	6,465
subtotal =	11,399.00																
Island =	2,153.99																
Phase 1 =	3,431.09																
Phase 2 =	1,102.76																
Phase 3 =	4,711.16																
unit price =	\$ 16.03	\$/m3															
Unit Rate (neat volume) =	\$ 19.23	\$/m3															

Sample calculations for cost of constructing foundation of the gateway structure

Description	Qty	Unit	Labour			Material	Equipment		Subcontract		Labour	Material	Equip.	Sub.	Total
			man hr	total mh	Rate	Rate	Qty	Rate	Qty	Rate					
Columns and Arch are pre-cast				-											
Concrete Formwork															
form & strip				-											
strip footings	4	m2	1.00	4	41.30	30.00	1	15.00			165	120	60	-	345
grind & patch	1	m2	0.10	0	41.30	2.00					4	2	-	-	6
pour concrete															
footings, foundation walls	5	m3	1.00	5	41.30						206	-	-	-	206
Concrete Reinforcing															
supply and install reinforcing steel	1	sub		-					1	230	-	-	-	230	230
Cast-in-Place Concrete															
foundations	5	m3		-		142.00					-	710	-	-	710
waste	2.5%	lot		-		710					-	18	-	-	18
pumping															
pump hours	1	hrs		-				1	165		-	-	165	-	165
meter charge	3	m3		-				1	4.00		-	-	11	-	11
pump travel	1	trips		-				1	165		-	-	165	-	165
Concrete Finishing															
N/A	-	m2		-											
Subtotal											375	850	401	230	1,856

Sample calculations for first costs of painting and signage

First Cost			
	Length (m)	\$/m	Total \$
Bike Lane Paint	160.0	80.0	12800.0
Curb and Gutter	267.0	70.0	18690.0
Phase 1 =	80.0	70.0	5600.0
Phase 2 =	64.0	70.0	4480.0
Phase 3 =	123.0	70.0	8610.0
	\$/ea	ea	Total \$
Road Marking	180.0	16.0	2880.0
Bike Lane Signs	300.0	6.0	1800.0
	\$/m2	m2	Total \$
Final Asphalt	5.0	1077.5	5387.6

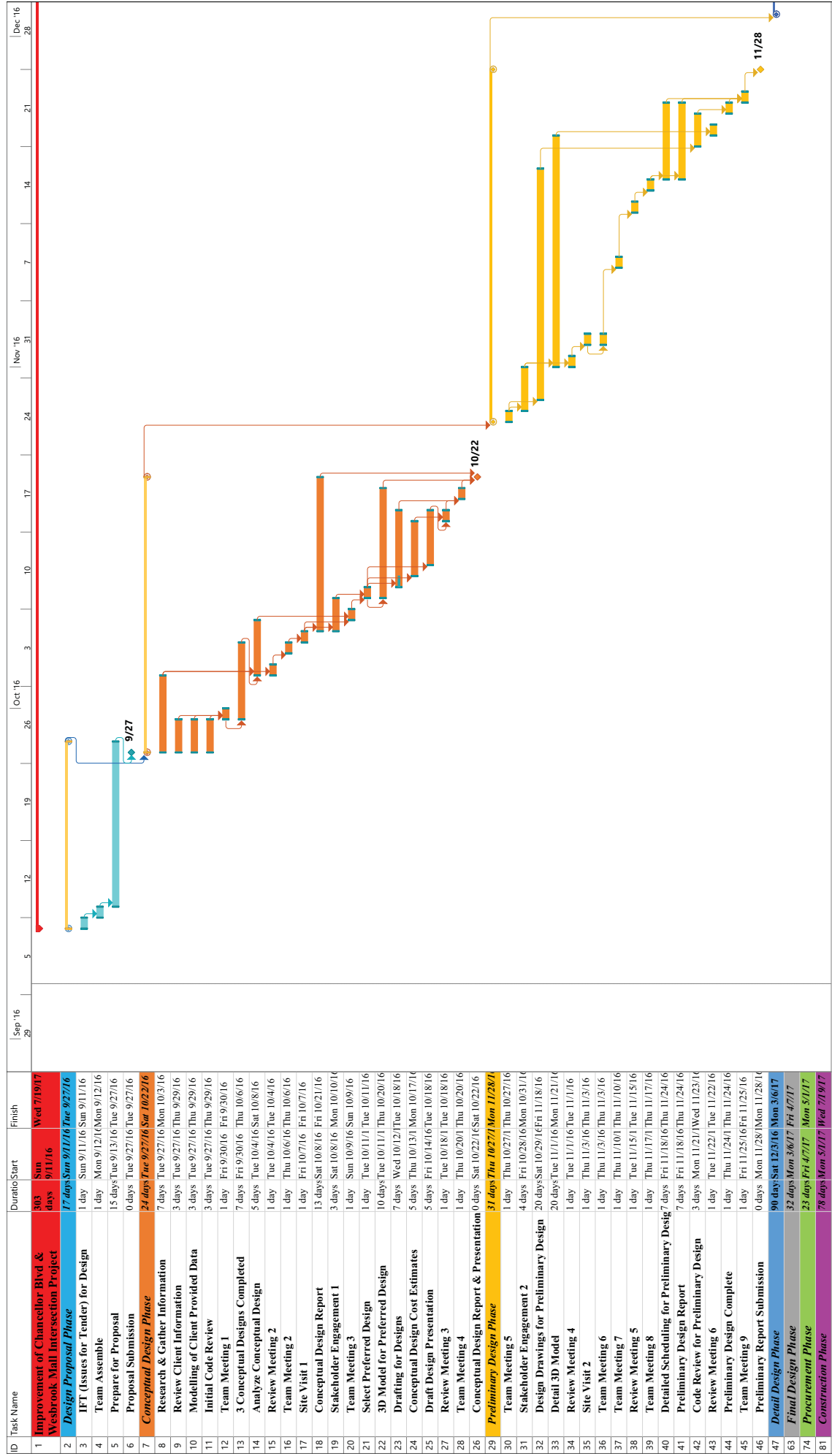
Sample calculations for maintenance cost of painting and signage

Maintenance (Redo every 10 yrs)				
	Length (m)	\$/m	Total \$	\$/yr
Bike Lane Paint	561.0	80.0	44880.0	4488.0
	\$/ea	ea	Total \$	\$/yr
Road Marking	180.0	17.0	3060.0	306.0

Permit Costs

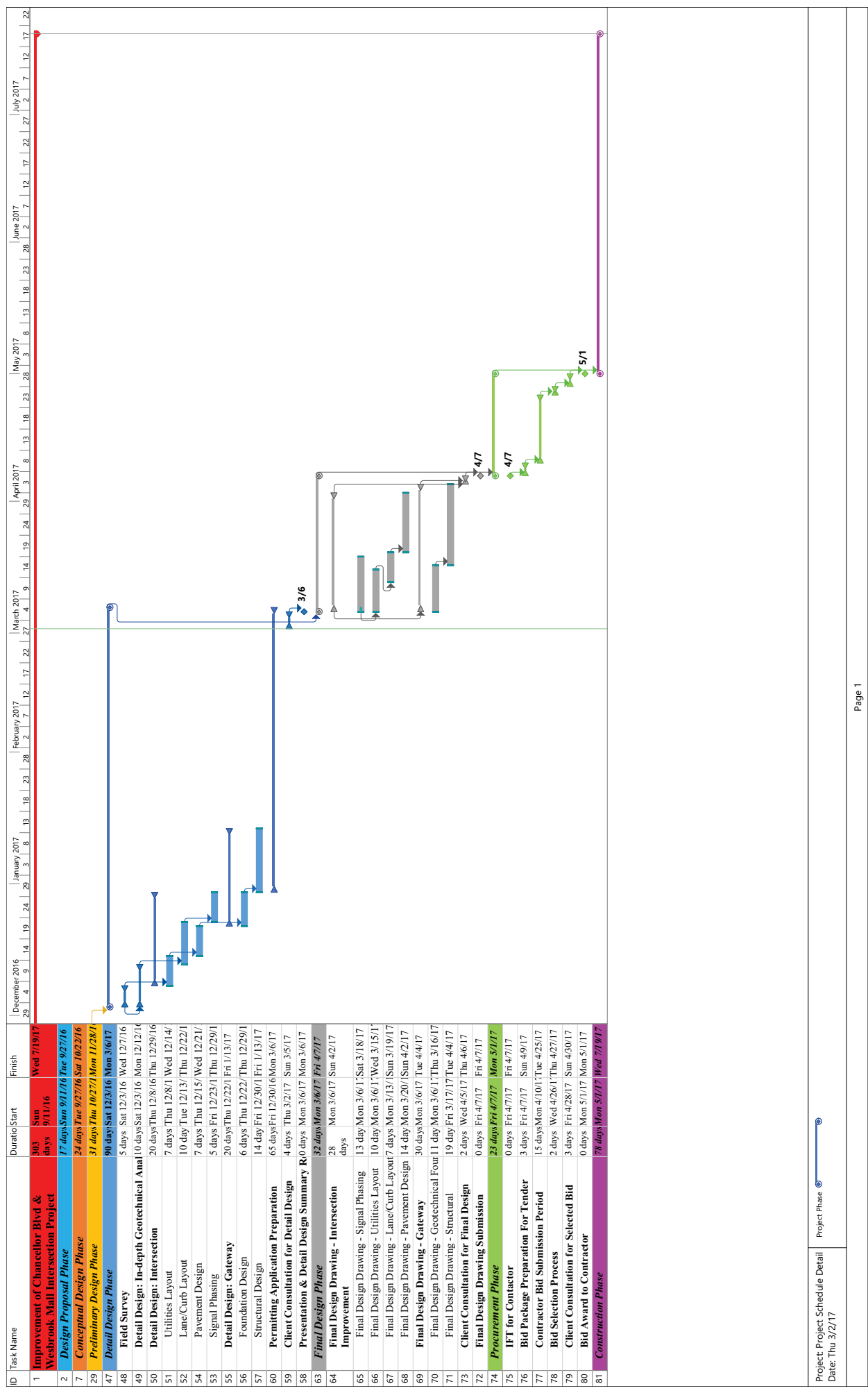
Development Permits		Area (100m ²)	Unit Cost (Dollars/100m ²)	Cost (Dollars)
Minor Applications			1500	1500
	Subtotal			1500
Major Applications	5		300	1500
	10		110	1100
	Subtotal			2600
Site Changes	Area (100m ²)		Unit Cost	Cost
	10		125	1250
	5		85	425
	Subtotal			1675
Alterations	Area (100m ²)		Unit Cost	Cost
	1		290	290
	Subtotal			290
Revisions	Subtotal			160
Total Cost				6225
Plumbing Permits			Quantity	Cost (Dollars)
	First 30 m		1	35
	Additional Length (240m)		8	160
	Backflow Prevention Device		3	225
	Watermain tie-ins/valve installation		1	75
	Sanitary/storm sewer tie-ins		1	75
	Manhole installation		6	0
	Catch Basin		5	0
	Firelines and Hydrants		2	52
Total Cost				622
Construction Noise Regulation - Variance Permit				148
Total Permit Cost (UBC)				6995
Utility Permit				660
Sidewalks and Landscaping Permit				200
Total Permit Cost (Municipal and Provincial)				860
Final Permit Cost				7855

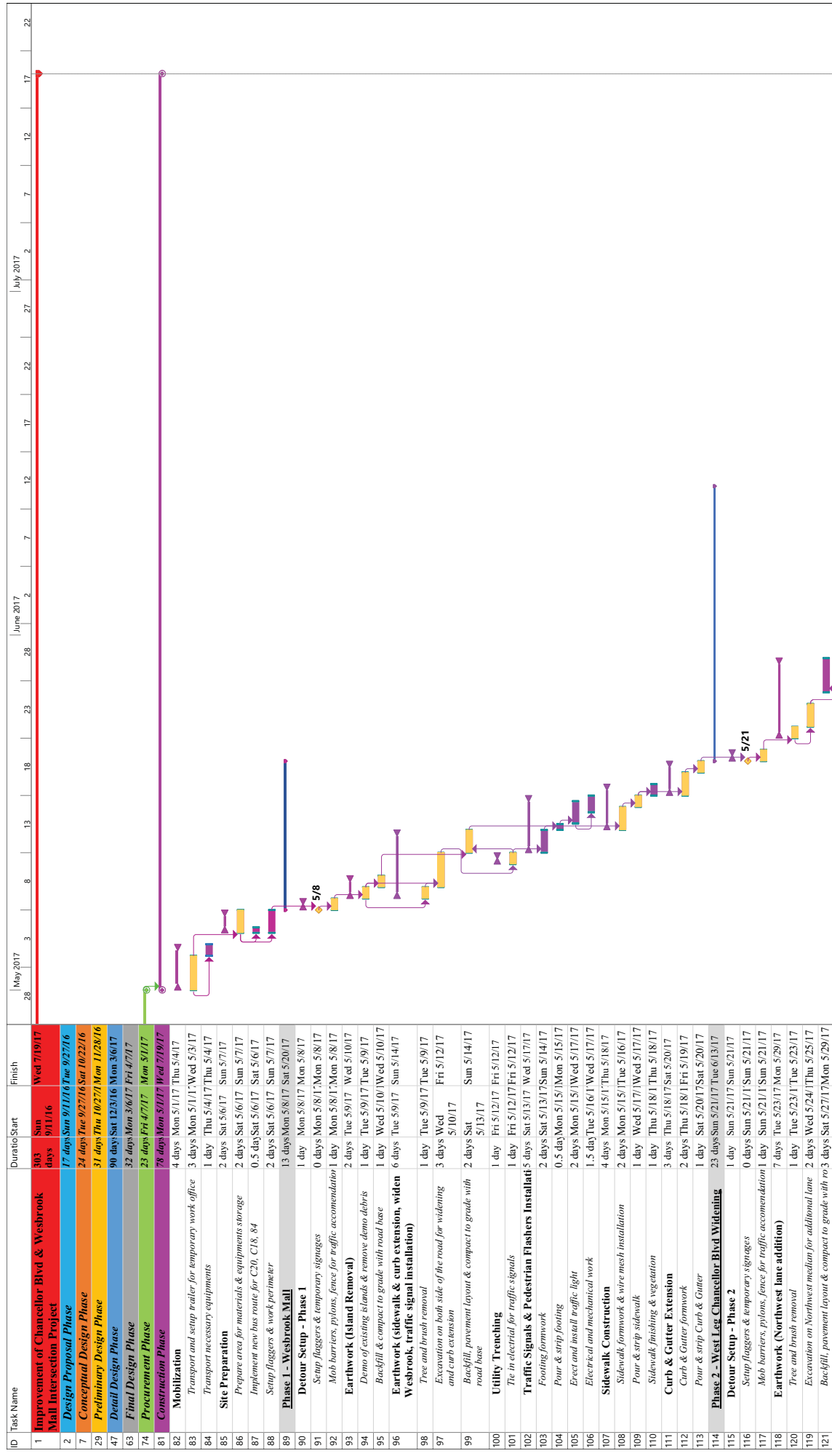
APPENDIX C : SCHEDULING

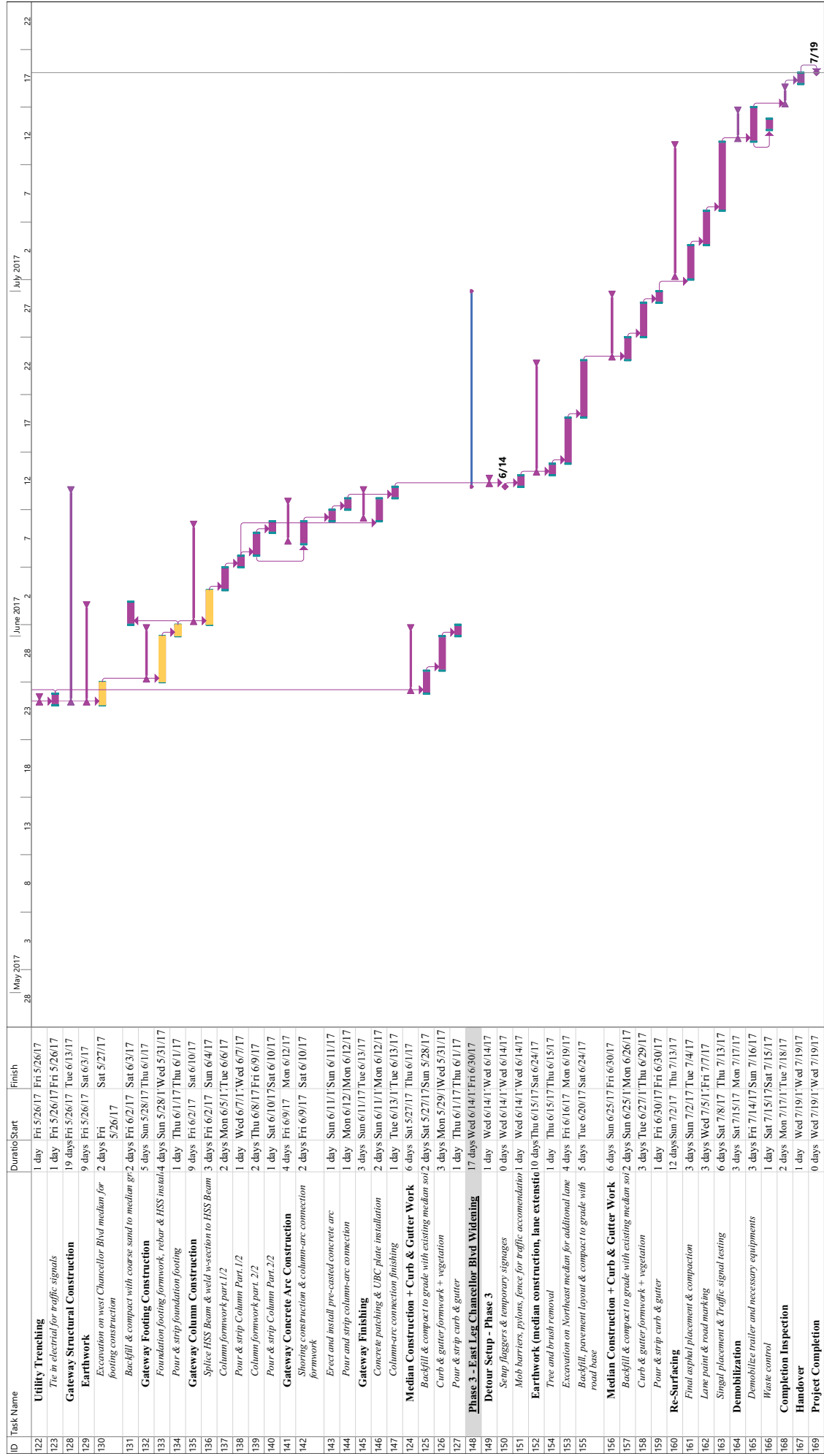


Project Project Schedule Detail
Date: Thu 3/2/17

Project Phase







APPENDIX D : SYNCHRO AND SIMTRAFFIC REPORTS

Lanes, Volumes, Timings

1: Int

3/27/2017

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↗		↖	↖↗	↖↗	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)		0.0	90.0		0.0	0.0
Storage Lanes		0	1		1	0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (m)	15.2		15.2	15.2	15.2	
Trailing Detector (m)	0.0		0.0	0.0	0.0	
Turning Speed (k/h)		14	24		24	14
Lane Util. Factor	1.00	1.00	0.91	0.91	1.00	1.00
Ped Bike Factor	1.00		1.00	1.00	0.98	
Fr _t	0.976				0.889	
Fl _t Protected			0.950	0.985	0.991	
Satd. Flow (prot)	1797	0	1597	3312	1589	0
Fl _t Permitted			0.370	0.795	0.991	
Satd. Flow (perm)	1797	0	620	2670	1588	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	31				258	
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Link Speed (k/h)	50			50	50	
Link Distance (m)	223.5			129.8	293.8	
Travel Time (s)	16.1			9.3	21.2	
Volume (vph)	373	82	98	109	54	248
Confl. Peds. (#/hr)		8	8		3	3
Confl. Bikes (#/hr)		1				5
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	389	85	102	114	56	258
Lane Group Flow (vph)	474	0	51	165	314	0
Turn Type			Perm			
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phases	4		8	8	2	
Minimum Initial (s)	4.0		4.0	4.0	4.0	
Minimum Split (s)	21.0		21.0	21.0	21.0	
Total Split (s)	24.0	0.0	24.0	24.0	21.0	0.0
Total Split (%)	53.3%	0.0%	53.3%	53.3%	46.7%	0.0%
Maximum Green (s)	19.0		19.0	19.0	16.0	
Yellow Time (s)	4.0		4.0	4.0	4.0	
All-Red Time (s)	1.0		1.0	1.0	1.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	
Recall Mode	None		None	None	Max	
Walk Time (s)	5.0		5.0	5.0	5.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	
Pedestrian Calls (#/hr)	8		3	3	3	
Act Effct Green (s)	15.3		15.3	15.3	17.2	
Actuated g/C Ratio	0.38		0.38	0.38	0.42	
v/c Ratio	0.68		0.22	0.16	0.38	
Control Delay	15.1		10.7	8.3	4.3	

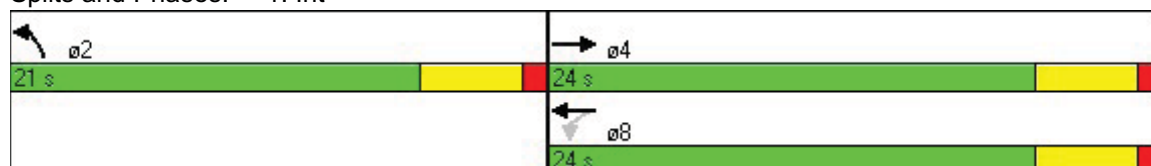


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	15.1		10.7	8.3	4.3	
LOS	B		B	A	A	
Approach Delay	15.1			8.9	4.3	
Approach LOS	B			A	A	

Intersection Summary

Area Type:	Other
Cycle Length:	45
Actuated Cycle Length:	40.6
Natural Cycle:	45
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.68
Intersection Signal Delay:	10.4
Intersection LOS:	B
Intersection Capacity Utilization	57.2%
ICU Level of Service	B
Analysis Period (min)	15

Splits and Phases: 1: Int



Lanes, Volumes, Timings

1: Int

3/27/2017

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↗		↖	↖↗	↖↗	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)		0.0	90.0		0.0	0.0
Storage Lanes		0	1		1	0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (m)	15.2		15.2	15.2	15.2	
Trailing Detector (m)	0.0		0.0	0.0	0.0	
Turning Speed (k/h)		14	24		24	14
Lane Util. Factor	1.00	1.00	0.91	0.91	1.00	1.00
Ped Bike Factor	1.00		1.00	1.00	0.97	
Fr _t	0.976				0.889	
Fl _t Protected			0.950	0.985	0.991	
Satd. Flow (prot)	1796	0	1597	3312	1584	0
Fl _t Permitted			0.297	0.749	0.991	
Satd. Flow (perm)	1796	0	498	2516	1583	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	32				294	
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Link Speed (k/h)	50			50	50	
Link Distance (m)	223.5			129.8	293.8	
Travel Time (s)	16.1			9.3	21.2	
Volume (vph)	465	102	122	136	68	310
Confl. Peds. (#/hr)		10	10		4	4
Confl. Bikes (#/hr)		1				6
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	484	106	127	142	71	323
Lane Group Flow (vph)	590	0	64	205	394	0
Turn Type			Perm			
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phases	4		8	8	2	
Minimum Initial (s)	4.0		4.0	4.0	4.0	
Minimum Split (s)	21.0		21.0	21.0	21.0	
Total Split (s)	29.0	0.0	29.0	29.0	21.0	0.0
Total Split (%)	58.0%	0.0%	58.0%	58.0%	42.0%	0.0%
Maximum Green (s)	24.0		24.0	24.0	16.0	
Yellow Time (s)	4.0		4.0	4.0	4.0	
All-Red Time (s)	1.0		1.0	1.0	1.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	
Recall Mode	None		None	None	Max	
Walk Time (s)	5.0		5.0	5.0	5.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	
Pedestrian Calls (#/hr)	10		4	4	4	
Act Effct Green (s)	18.9		18.9	18.9	17.2	
Actuated g/C Ratio	0.43		0.43	0.43	0.39	
v/c Ratio	0.75		0.30	0.19	0.49	
Control Delay	16.5		12.0	7.8	6.3	

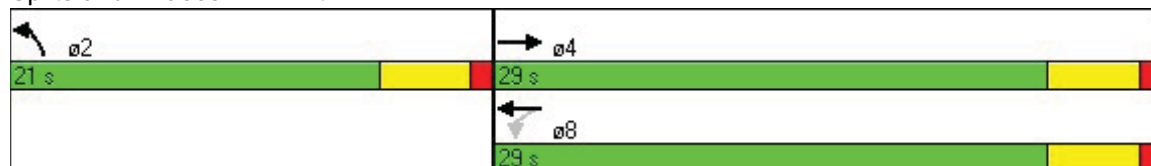


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	16.5		12.0	7.8	6.3	
LOS	B		B	A	A	
Approach Delay	16.5			8.8	6.3	
Approach LOS	B			A	A	

Intersection Summary

Area Type:	Other
Cycle Length:	50
Actuated Cycle Length:	44.3
Natural Cycle:	50
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.75
Intersection Signal Delay:	11.6
Intersection LOS:	B
Intersection Capacity Utilization	69.0%
ICU Level of Service	C
Analysis Period (min)	15

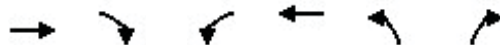
Splits and Phases: 1: Int



Lanes, Volumes, Timings

1: Int

3/27/2017



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)		0.0	90.0		0.0	0.0
Storage Lanes		0	1		1	0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (m)	15.2		15.2	15.2	15.2	
Trailing Detector (m)	0.0		0.0	0.0	0.0	
Turning Speed (k/h)		14	24		24	14
Lane Util. Factor	1.00	1.00	0.91	0.91	1.00	1.00
Ped Bike Factor	0.99				0.97	
Frt	0.976				0.889	
Flt Protected			0.950	0.985	0.991	
Satd. Flow (prot)	1793	0	1597	3312	1579	0
Flt Permitted			0.140	0.598	0.991	
Satd. Flow (perm)	1793	0	235	2010	1577	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	26				197	
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Link Speed (k/h)	50			50	50	
Link Distance (m)	223.5			129.8	293.8	
Travel Time (s)	16.1			9.3	21.2	
Volume (vph)	648	142	171	190	94	432
Confl. Peds. (#/hr)		14	14		5	5
Confl. Bikes (#/hr)		1				6
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	675	148	178	198	98	450
Lane Group Flow (vph)	823	0	89	287	548	0
Turn Type			Perm			
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phases	4		8	8	2	
Minimum Initial (s)	4.0		4.0	4.0	4.0	
Minimum Split (s)	21.0		21.0	21.0	21.0	
Total Split (s)	39.0	0.0	39.0	39.0	26.0	0.0
Total Split (%)	60.0%	0.0%	60.0%	60.0%	40.0%	0.0%
Maximum Green (s)	34.0		34.0	34.0	21.0	
Yellow Time (s)	4.0		4.0	4.0	4.0	
All-Red Time (s)	1.0		1.0	1.0	1.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	
Recall Mode	None		None	None	Max	
Walk Time (s)	5.0		5.0	5.0	5.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	
Pedestrian Calls (#/hr)	5		5	5	14	
Act Effct Green (s)	30.9		30.9	30.9	22.2	
Actuated g/C Ratio	0.50		0.50	0.50	0.36	
v/c Ratio	0.90		0.75	0.28	0.79	
Control Delay	27.5		53.3	9.2	22.3	

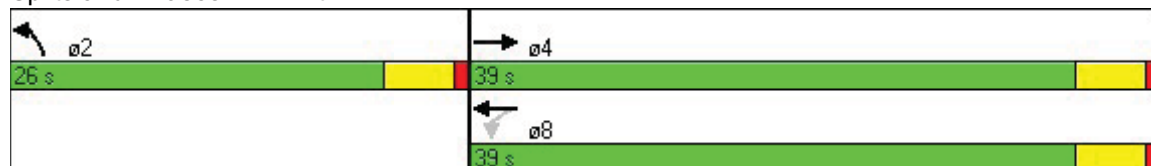


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	27.5		53.3	9.2	22.3	
LOS	C		D	A	C	
Approach Delay	27.5			19.7	22.3	
Approach LOS	C			B	C	

Intersection Summary

Area Type:	Other
Cycle Length:	65
Actuated Cycle Length:	61.2
Natural Cycle:	65
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.90
Intersection Signal Delay:	24.2
Intersection LOS:	C
Intersection Capacity Utilization:	92.1%
ICU Level of Service:	F
Analysis Period (min):	15

Splits and Phases: 1: Int



Lanes, Volumes, Timings

1: Int

3/27/2017

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↗		↘	↖↗	↖↗	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)		0.0	90.0		0.0	0.0
Storage Lanes		0	1		1	0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (m)	15.2		15.2	15.2	15.2	
Trailing Detector (m)	0.0		0.0	0.0	0.0	
Turning Speed (k/h)		14	24		24	14
Lane Util. Factor	1.00	1.00	0.91	0.91	1.00	1.00
Ped Bike Factor	1.00		1.00	1.00	0.98	
Fr _t	0.976				0.889	
Fl _t Protected			0.950	0.985	0.991	
Satd. Flow (prot)	1797	0	1597	3312	1589	0
Fl _t Permitted			0.370	0.795	0.991	
Satd. Flow (perm)	1797	0	620	2670	1588	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	31				258	
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Link Speed (k/h)	50			50	50	
Link Distance (m)	223.5			129.8	293.8	
Travel Time (s)	16.1			9.3	21.2	
Volume (vph)	373	82	98	109	54	248
Confl. Peds. (#/hr)		8	8		3	3
Confl. Bikes (#/hr)		1				5
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	389	85	102	114	56	258
Lane Group Flow (vph)	474	0	51	165	314	0
Turn Type			Perm			
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phases	4		8	8	2	
Minimum Initial (s)	4.0		4.0	4.0	4.0	
Minimum Split (s)	21.0		21.0	21.0	21.0	
Total Split (s)	24.0	0.0	24.0	24.0	21.0	0.0
Total Split (%)	53.3%	0.0%	53.3%	53.3%	46.7%	0.0%
Maximum Green (s)	19.0		19.0	19.0	16.0	
Yellow Time (s)	4.0		4.0	4.0	4.0	
All-Red Time (s)	1.0		1.0	1.0	1.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	
Recall Mode	None		None	None	Max	
Walk Time (s)	5.0		5.0	5.0	5.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	
Pedestrian Calls (#/hr)	8		3	3	3	
Act Effct Green (s)	15.3		15.3	15.3	17.2	
Actuated g/C Ratio	0.38		0.38	0.38	0.42	
v/c Ratio	0.68		0.22	0.16	0.38	
Control Delay	15.1		10.7	8.3	4.3	

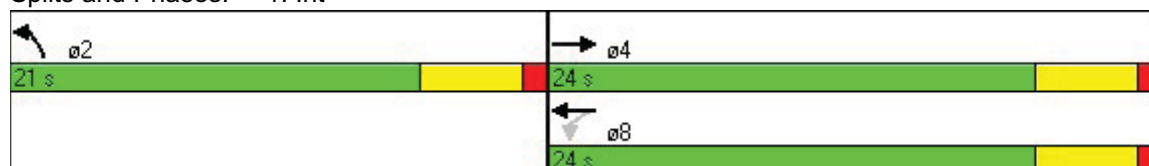


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	15.1		10.7	8.3	4.3	
LOS	B		B	A	A	
Approach Delay	15.1			8.9	4.3	
Approach LOS	B			A	A	

Intersection Summary

Area Type:	Other
Cycle Length:	45
Actuated Cycle Length:	40.6
Natural Cycle:	45
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.68
Intersection Signal Delay:	10.4
Intersection LOS:	B
Intersection Capacity Utilization	57.2%
ICU Level of Service	B
Analysis Period (min)	15

Splits and Phases: 1: Int



Lanes, Volumes, Timings

1: Int

3/27/2017

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↗		↘	↖	↗	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)		0.0	90.0		0.0	0.0
Storage Lanes		0	1		1	0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (m)	15.2		15.2	15.2	15.2	
Trailing Detector (m)	0.0		0.0	0.0	0.0	
Turning Speed (k/h)		14	24		24	14
Lane Util. Factor	1.00	1.00	0.91	0.91	1.00	1.00
Ped Bike Factor	1.00		1.00	1.00	0.97	
Frt	0.976				0.889	
Flt Protected			0.950	0.985	0.991	
Satd. Flow (prot)	1796	0	1597	3312	1584	0
Flt Permitted			0.297	0.749	0.991	
Satd. Flow (perm)	1796	0	498	2516	1583	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	32				294	
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Link Speed (k/h)	50			50	50	
Link Distance (m)	223.5			129.8	293.8	
Travel Time (s)	16.1			9.3	21.2	
Volume (vph)	465	102	122	136	68	310
Confl. Peds. (#/hr)		10	10		4	4
Confl. Bikes (#/hr)		1				6
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	484	106	127	142	71	323
Lane Group Flow (vph)	590	0	64	205	394	0
Turn Type			Perm			
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phases	4		8	8	2	
Minimum Initial (s)	4.0		4.0	4.0	4.0	
Minimum Split (s)	21.0		21.0	21.0	21.0	
Total Split (s)	29.0	0.0	29.0	29.0	21.0	0.0
Total Split (%)	58.0%	0.0%	58.0%	58.0%	42.0%	0.0%
Maximum Green (s)	24.0		24.0	24.0	16.0	
Yellow Time (s)	4.0		4.0	4.0	4.0	
All-Red Time (s)	1.0		1.0	1.0	1.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	
Recall Mode	None		None	None	Max	
Walk Time (s)	5.0		5.0	5.0	5.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	
Pedestrian Calls (#/hr)	10		4	4	4	
Act Effct Green (s)	18.9		18.9	18.9	17.2	
Actuated g/C Ratio	0.43		0.43	0.43	0.39	
v/c Ratio	0.75		0.30	0.19	0.49	
Control Delay	16.5		12.0	7.8	6.3	

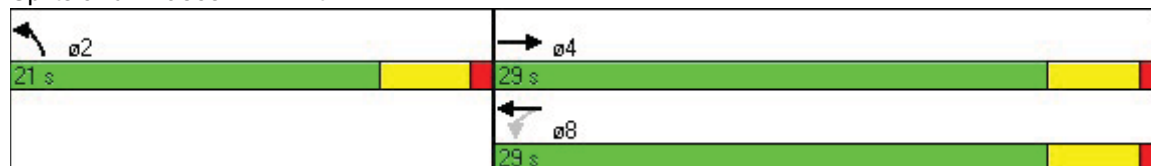


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	16.5		12.0	7.8	6.3	
LOS	B		B	A	A	
Approach Delay	16.5			8.8	6.3	
Approach LOS	B			A	A	

Intersection Summary

Area Type:	Other
Cycle Length:	50
Actuated Cycle Length:	44.3
Natural Cycle:	50
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.75
Intersection Signal Delay:	11.6
Intersection LOS:	B
Intersection Capacity Utilization	69.0%
ICU Level of Service	C
Analysis Period (min)	15

Splits and Phases: 1: Int



Lanes, Volumes, Timings

1: Int

3/27/2017

	→	↘	↙	←	↖	↗
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↗		↖	↗↖	↖↗	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)		0.0	90.0		0.0	0.0
Storage Lanes		0	1		1	0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (m)	15.2		15.2	15.2	15.2	
Trailing Detector (m)	0.0		0.0	0.0	0.0	
Turning Speed (k/h)		14	24		24	14
Lane Util. Factor	1.00	1.00	0.91	0.91	1.00	1.00
Ped Bike Factor	0.99				0.97	
Frt	0.976				0.889	
Flt Protected			0.950	0.985	0.991	
Satd. Flow (prot)	1793	0	1597	3312	1579	0
Flt Permitted			0.140	0.598	0.991	
Satd. Flow (perm)	1793	0	235	2010	1577	0
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	26				197	
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Link Speed (k/h)	50			50	50	
Link Distance (m)	223.5			129.8	293.8	
Travel Time (s)	16.1			9.3	21.2	
Volume (vph)	648	142	171	190	94	432
Confl. Peds. (#/hr)		14	14		5	5
Confl. Bikes (#/hr)		1				6
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	675	148	178	198	98	450
Lane Group Flow (vph)	823	0	89	287	548	0
Turn Type			Perm			
Protected Phases	4			8	2	
Permitted Phases			8			
Detector Phases	4		8	8	2	
Minimum Initial (s)	4.0		4.0	4.0	4.0	
Minimum Split (s)	21.0		21.0	21.0	21.0	
Total Split (s)	39.0	0.0	39.0	39.0	26.0	0.0
Total Split (%)	60.0%	0.0%	60.0%	60.0%	40.0%	0.0%
Maximum Green (s)	34.0		34.0	34.0	21.0	
Yellow Time (s)	4.0		4.0	4.0	4.0	
All-Red Time (s)	1.0		1.0	1.0	1.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	
Recall Mode	None		None	None	Max	
Walk Time (s)	5.0		5.0	5.0	5.0	
Flash Dont Walk (s)	11.0		11.0	11.0	11.0	
Pedestrian Calls (#/hr)	5		5	5	14	
Act Effct Green (s)	30.9		30.9	30.9	22.2	
Actuated g/C Ratio	0.50		0.50	0.50	0.36	
v/c Ratio	0.90		0.75	0.28	0.79	
Control Delay	27.5		53.3	9.2	22.3	

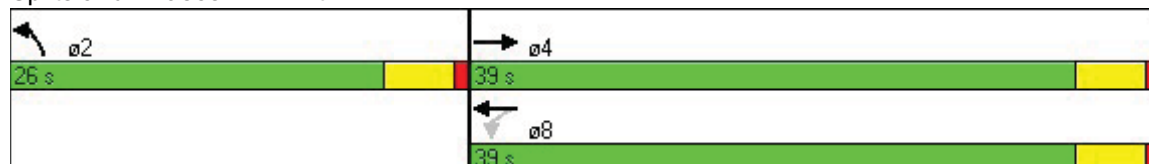


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Queue Delay	0.0		0.0	0.0	0.0	
Total Delay	27.5		53.3	9.2	22.3	
LOS	C		D	A	C	
Approach Delay	27.5			19.7	22.3	
Approach LOS	C			B	C	

Intersection Summary

Area Type:	Other
Cycle Length:	65
Actuated Cycle Length:	61.2
Natural Cycle:	65
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.90
Intersection Signal Delay:	24.2
Intersection LOS:	C
Intersection Capacity Utilization:	92.1%
ICU Level of Service:	F
Analysis Period (min):	15

Splits and Phases: 1: Int



APPENDIX E : STRUCTURAL CALCULATIONS

Calculation Date:

2017-03-02

Calculation of Dead Loads

HSS Columns: 305x305x13

Length m
 Density kN/m

Dead Load: 13.32 kN

Concrete Arch:

Volume m³
 Density kN/m³

Dead Load: 26.62 kN

Concrete Cover:

Volume m³
 Density kN/m³

Dead Load: 21.62 kN

Steel Plates: 50 mm plates

Area m²
 Density kN/m²

Dead Load: 77 kN

W-Shape W310x375

Length m
 Density kN/m

Dead Load: 92 kN

Total Dead Load: 230.56 kN

Earthquake Overturning Force

Component	Dead Load	DL*min.EQ force	Center of mass (m)
Arch	26.62 KN	6.015	6 36.09 knM
Plate and Column	203.9416 KN	46.09	2 92.18 knM
			128.27 knM

Calculation of Earthquake Loads and Effects (Section 4.1.8. of NBC 2010)

Input Parameters:

Importance Category:	Normal	→	I_E :	1.0	(NBCC 2010 Table 4.1.8.5)
Province:	British Columbia	→	$S_a(0.2)$:	0.880	
Site Location:	West Vancouver		$S_a(0.5)$:	0.620	
			$S_a(1.0)$:	0.330	
			$S_a(2.0)$:	0.170	
			PGA:	0.430	(NBCC Table C-2)
Site Class:	C	→	F_a :	1.0	(NBCC 2010 Table 4.1.8.4.B.)
SFRS Material:	Steel		F_v :	1.0	(NBCC 2010 Table 4.1.8.4.C.)
SFRS Type:	Limited ductility moment-resisting frames	→	R_d :	2.0	
			R_o :	1.3	(NBCC 2010 Table 4.1.8.9)
Height of Structure	7.98	m			
Number of storeys	1				
Type of SFRS	Other systems	→	T_a :	0.237	(NBCC 2010 Cl. 4.1.8.11. (3))
			M_v :	1.0	(NBCC 2010 Table 4.1.8.11)
			J:	1.0	(NBCC 2010 Table 4.1.8.11)
			S(T):	0.8475907	(NBCC 2010 Cl. 4.1.8.4.(7))
Weight of Structure	1.0	kN			(Use 1.0kN to get seismic load as a percentage of weight)

Calculation of Minimum Lateral Earthquake Force:

Minimum lateral earthquake force, V
 $V = S(T_a)M_vI_EW/(R_dR_o) = 0.326$ (NBCC Clause 4.1.8.11.(2))

Lower bound of V for moment-resisting frames, braced frames, and other systems
 $S(2.0)M_vI_EW/(R_dR_o) = 0.065$ (NBCC Clause 4.1.8.11.(2)b)

Upper bound of V for buildings NOT on site Class F and with $R_d \geq 1.5$
 $(2/3)S(2.0)I_EW/(R_dR_o) = 0.226$ (NBCC Clause 4.1.8.11.(2)c) **(Governs)**

Therefore, the minimum lateral earthquake force used for analysis is: 0.225641026
 (as a % of weight)

Calculation of Specified Wind Load (Cl. 4.1.7.1. of NBC 2010)

Input Parameters:

Importance Category:	Normal	→	$I_w = 1.00$ (ULS) (NBCC 2010 Table 4.1.7.1)
			$I_w = 0.75$ (SLS) (NBCC 2010 Table 4.1.7.1)
Province:	British Columbia		
Site Location:	West Vancouver	→	$p = 0.48$ kPa (NBCC Table C-2)
Terrain:	Rough		
Elev _{top of roof} :	7.98		
Elev _{top of wall} :	7.98		
Elev _{top of grade} :	0.000		
Reference height (h):	7.98 m	→	$C_e = 0.70$ (Par. 7, Commentary I, NBC 2010 Structural Commentaries)
			(NBCC 2010 Clause 4.1.7.1.(5))
Roof Slope:	0		
α :	0		

Components and Cladding - Walls:

Consider the arch as a wall/cladding structure and use the appropriate table in the Structural Commentary

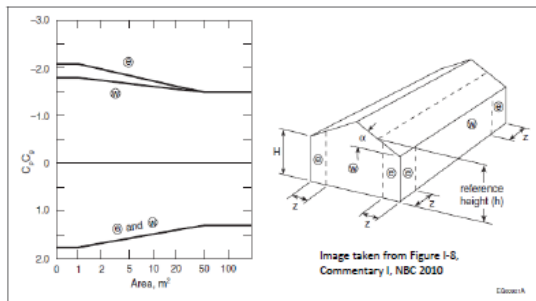
Ds:	5.82	m	(Least horizontal dimension)
H:	7.980	m	(H is defined above in image for Load Case A)
End zone width z:	1.0	m	(Note 7, Figure I-7, Commentary I, NBC 2010 Structural Commentaries)
End zone width y:	6	m	(Note 6, Figure I-7, Commentary I, NBC 2010 Structural Commentaries)

Components and Cladding - Walls:

L_{wall}	5.82	m	(Length of wall)
L_{gable}	5.82	m	(Length of gable)
z	1.0	m	
H	7.980	m	
h	7.98	m	

	Area (m ²)	$C_p C_g$	p (kPa)	$C_p C_g$ (-)	p (kPa)
E_{wall}	8.0	1.53	0.51	-1.79	-0.60
W_{wall}	30.5	1.37	0.46	-1.58	-0.53
E_{gable}	8.0	1.53	0.51	-1.79	-0.60
W_{gable}	30.5	1.37	0.46	-1.58	-0.53

Use $p = 0.51$ kPa



Calculation of Specified Snow Load (Cl. 4.1.6.2. of NBC 2010)

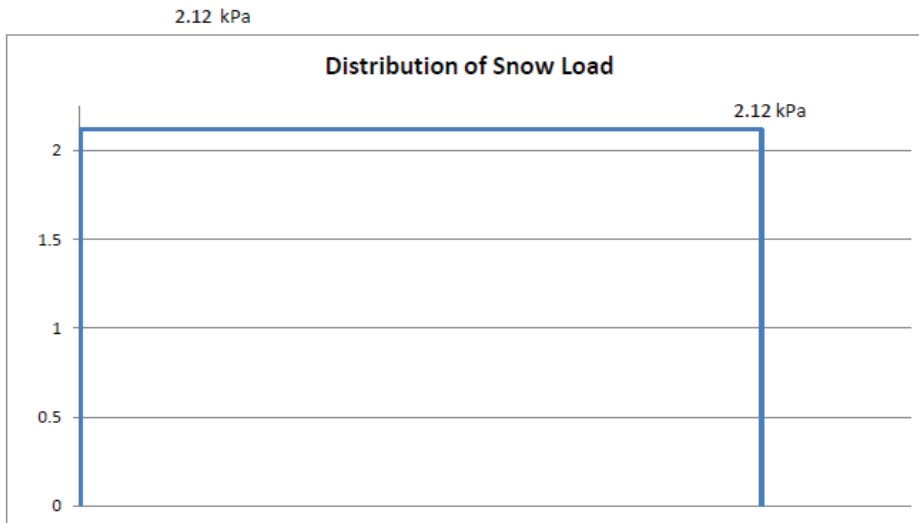
Input Parameters:

Importance Category:	Normal	→	I_s : 1.0 (ULS) (NBCC 2010 Table 4.1.6.2)
			I_s : 0.9 (SLS) (NBCC 2010 Table 4.1.6.2)
Province:	British Columbia	→	S_s : 2.4 kPa (NBCC Table C-2)
Site Location:	West Vancouver		S_r : 0.2 kPa (NBCC Table C-2)
shorter roof plan dimension = w =	0.41 m		C_w : 1.0 (NBCC Cl. 4.1.6.2 (3), (4))
longer roof plan dimension = l =	5.82 m		
$l_c = 2w - w^2/l =$	0.8 m	→	C_b : 0.8 (NBCC Cl. 4.1.6.2 (2))
$\alpha =$	0 °	→	C_s : 1.0 (NBCC Cl. 4.1.6.2 (5), (6))
Slippery Roof?	No		Case I Loading C_a : 1.0 (Par. 33, Commentary G, NBC 2010 Structural Commentaries)

Calculation and Graphical Display of Snow Load:

$$S = I_s [S_s (C_b C_w C_s C_a) + S_r]$$

Case I Loading (ULS):



Approximate the snow load as a distributed load. (This is a conservative overestimation, at the very edge of the arch, there is less snow due to the large slope)

Calculation Date: 2017-03-03

Structural Verification

Dead Load:	230.56	kN
Snow Load:	2.12	kPa
Wind Load:	0.51	kPa
Earthquake Load:	52.11	kN

Governing Load Case for Vertical Loads:
1.25DL + 1.5SL 295.79 kN

Governing Case for Lateral Shear (at Base):
1.0EQ 52.11 kN

Governing Case for Overturning
0.9DL + 1.4WL 12.59 kN

Bearing Capacity:

Each footing takes half of the gravity load

Area of footing: 3.65 m²
Load per footing 40.54 kPa

Soil is quadra sand, base of footing at 1.0 below grade

Unit weight of sand 19.8 kN/m³
Effective Unit Weight 10 kN/m³
From Meyerhof:

N_q 18.4
 N_y 15.7
Vertical effective stress 10 kPa
Bearing Capacity 303.948 kPa FOS= 7.5

Compressive Strength of HSS

From S16-09, page 4-50, the compressive strength for an HSS 305 x 305 x 13 with effective length 6m
 $C_r = 3920$ kN 4% of steel capacity used

W310x375 Check

Wind load causes bending on the plate:

Maximum moment: 0.6375 kNm (per W shape)
Moment Capacity: 2520 kNm (per 1 W310x375)
Deflection: 10.28 mm (per W shape)
Allowable Deflection 16.67 mm (L/300)

HSS Check

Shear Capacity: 1841 kN 3% of steel capacity used
Moment Capacity: 561.6 kNm 9% of steel capacity used

HSS Deflection:**Wind**

Deflection (per column) 0.35 mm
 h/300 check 20.00 mm

Earthquake

Deflection (bottom 1m) 0.00 mm
 Deflection (top 3m) 0.09 mm
Total Deflection 0.09 mm

Sliding Check

Friction factor between soil and concrete 0.5

Applied Force 52.11 kN
 Resisting (Sliding) Force 115.28 kN FOS= 2.21

Overturning Check

Wind load is distributed evenly over the height:

Moment arm of wind: 3.99 m
 Overturning Moment: 50.23 kNm
 Resisting Moment: 198.17 kNm FOS = 3.95

Earthquake Overturning

Overturning Moment

Component	Dead Load	EQ Force	Moment arm	Overturning
Arch	26.62 KN	6.02	6	36.092 knM
Plate and Column	203.9 KN	46.09	2	92.182 knM
				<u>128.27 knM</u>

Tipping

Overturning Moment 128.27 kNm
 Resisting Moment 843.85 kNm FOS= 6.58

Overturning out of Plane

Overturning Moment 128.27 kNm
 Resisting Moment 198.17 kNm FOS= 1.54

Check increased bearing on soil due to overturning moment:

$q_{max} = P/A + My/I$, where P/A is the bearing capacity calculated due to centric loading, and M is the greatest bending moment from the different load cases considered.

The greatest overturning moment is nearly 130 kNm

$I = Lh^3/12$, where $L = 1.91$ m and $h = 0.5$ m. y is the distance from the centroid to the farthest fibre, which is equal to $L/2 = 1.91$ m/2.

Plugging in the values, the stress from the bending moment is equal to 6.24 Pa, which is nearly negligible compared to the bearing capacity of the axial load.

Steel Connection Design:

Fillet Weld connection for the W-shape to HSS

Length of weld is 330 mm

Using CSA S16-09, Cl. 13.13.2.2.

$$V_r = 0.67\phi_w A_w X_u (1.00 + 0.50 \sin^{1.5} \theta) M_w$$

$$\phi_w = 0.67$$

$$\theta = 0^\circ \text{ (longitudinal weld)}$$

$$M_w = 1.0 \text{ (single weld orientation)}$$

Equal leg lengths of 10 mm, throat size = 7.071 mm

$$\text{Then } A_w = 7.071 \times 330 = 2333.43 \text{ mm}^2$$

$$X_u = 490 \text{ MPa (G40.21 Grade 300)}$$

$$V_r = 513.3 \text{ kN}$$

This is the shear capacity of one weld, there will be a total of 20 welds for 5 W-shapes, and the shear capacity is much higher than the self weight of the beams and plate, therefore, the connection is sufficient.

Bolt connection of Plate to W – shape

Check the tensile capacity of the steel plate:

T_r is the minimum of the following:

$$T_r = \phi A_g F_y$$

$$T_r = \phi_u A_{ne} F_u$$

$$T_r = 0.75 [U_t A_n F_u + 0.6 (F_y + F_u / 2) A_{gv}]$$

$$T_r = \phi A_g F_y = 0.9 \times 50 \times 5000 \times 350 = 78750 \text{ kN}$$

Use 50 24 dia. bolts spaced 100 mm apart

$$\text{Then } T_r = \phi_u A_{ne} F_u = 769.5 \text{ kN, using } A_{ne} = 0.6 A_n.$$

$$\text{Finally, using edge distance 30 mm, } T_r = 0.75 [U_t A_n F_u + 0.6 (F_y + F_u / 2) A_{gv}] = 1822.5 \text{ kN}$$

Therefore the tensile resistance is more 769.5 kN which is much greater than the self weight of the plates, and the connection is sufficient.

Check the shear resistance of the bolts: the unit factored shear resistance of an A325M bolt is 279 MPa, assuming the threads are intercepted.

The area of one 24 dia. bolt is 452 mm², and there are a total of 100 bolts on each side. Thus the shear capacity is 12610 kN using 50 bolts.

Modify the design to use only 2 bolts on the bottom of the plate at either end, and decrease the number of bolts along the top to have 10 bolts spaced at 500 mm.

Then shear capacity of the connection is 1514 kN. The tensile capacity of the plate will increase as the number of bolts decrease, so no need to check again.

Footing Detailed Calculations

Given the following parameters:

Concrete cover = 75 mm

15M reinforcement

$b = l = 1.91 \text{ m}$

$h = 0.5 \text{ m}$

Load per footing = 148 kN = P_f

$q_f = 41 \text{ kPa}$

Calculate effective depth:

$$d = h - \text{cover} - d_b/2 = 500 - 75 - 15/2 = 415 \text{ mm}$$

$$\text{Critical Perimeter} = b_0 = 4(t+d) = 4(410+415) = 3300\text{mm}$$

Then $V_c = v_c \times b_0 \times d$, where $v_c = 0.38\lambda\phi_c\sqrt{f'_c}$

$$v_c = 0.38(1)(0.65)(\sqrt{30}) = 1.35$$

$$V_c = v_c \times b_0 \times d = 1.35 \times 3300 \times 415 = 1849 \text{ kN}$$

$$V_f = q_f [A - (t+d)^2] = 41[1910^2 - (410+415)^2] = 122 \text{ kN}$$

Since $V_c \gg V_f$, then shear reinforcement is not required

Factored Bending Moment is given by the following equation:

$$M_f = q_f \times ((l-t)/2) \times ((l-t)/4) \times b = 22 \text{ kN}$$

$$\text{Set } M_r = M_f, \text{ then use } A_s = 0.0015f'_c b(d - \sqrt{d^2 - (3.85M_r/(f'_c b))})$$

$$\text{Then } A_s = 155 \text{ mm}^2$$

$$A_{s\text{min}} = 0.002A_g = 1910 \text{ mm}^2, \text{ so use 10 15M bars}$$

$$\text{Required spacing} = A_b \times 1000 / A_s = 100 \text{ mm}$$

The reinforcement in both directions is identical since the footing is square. The reinforcement details can be found on the structural drawings.