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1.0 EXECUTIVE SUMMARY

Our team has been retained by UBC to develop a multi-use stormwater retention system reflecting the need to address the event of a 100 year return period storm. This call to action is as result of a need to address and prevent cliff erosion off the Southwest Marine Drive and Point Grey area. The system shall be situated adjacent to the UBC Center for Comparative Medicine (CCM) and shall encompass key design criteria as laid out by Mr. Doyle while adhering to UBC SEEDS Department's criteria to sustainability at UBC.

As result, our team is proposing a multi-use Velodrome design to combat the 100 year return period storm whilst also providing an excellent cornerstone for growth within the UBC community. The Velodrome will also be encompassing a park within it as to further push UBC SEEDS Department's criteria to sustainability and a greener image. The system will limit the outflow to approximately 1.2 m³/s. Additionally, the volumetric capacity will contain 3,400 cubic meters of water and fulfills the BC Water Quality Guidelines for the discharge of water.

Given the provided geological data, soil conditions and borehole data, our team has developed a design structure and ensuing calculations to support the development of the UBC Velodrome. Additionally, Issued for Construction (IFC) drawings are enclosed for the construction of this project.

Utilizing UBC, Metro Vancouver, Canadian, and other applicable guidelines, the construction processes are identified and a proposed schedule of the entire project is enclosed. Using RSMeans, other notable municipal and federal projects information, and finalized details in our service life and maintenance plan including the time value of money and interest rates, our Class C cost estimate has been updated to a Class B cost estimate. Additionally, our team will include considerations for the service life and maintenance of the UBC Velodrome as well as recommendations for its design.
2.0 DESIGN CRITERIA & ASSUMPTIONS

The key standards and software packages used in the design process include:

- National Building Code of Canada
- Canadian Foundation Engineering Manual (FEM)
- AASHTO
- ASTM
- Vancouver by-laws
- American Concrete Institute (ACI)
- CSA A23.3-14 (design of concrete structures)
- CSA A23.2-14 (test methods and standard practices for concrete)
- CSA A23.1-14 (concrete materials and methods for construction)
- CAC - Concrete Design Handbook
- Software: AutoCAD, Civil 3D

Key Design Issues:

The major design issues that were foreseen, and successfully addressed, are the design of the retaining wall and the mechanism to lay the slab (bicycle-track) on the retaining wall. Since there are no readily available codes and standards, specifically catering to the design of velodromes, a thorough analysis was required to ensure that all safeguards are in place. For this, the team referred to all standards and codes applicable referenced above.

Technical considerations associated with the design:

- Sizing of inlet and outlet stormwater pipes to control discharge
- Final design calculations
- Final cost estimate
- Final project schedule
- Detailed engineering drawings

Design criteria:

- Limited Flow Capacity: For the UBC south campus catchment, the storm-water retention structure to be located at Westbrook Mall and Marine Drive requires a limited 1.2 m3/s [1] output to avoid inundating the culverts crossing Marine Drive, and as such this was worked
into our design via the size of the piping for the water leaving the site. The quantity and quality of stormwater leaving campus was considered to be an important factor in our design, and was addressed by our multi-layer rain garden design which removes 90% of nutrients and chemicals and 80% of sediments from rainfall catchment prior to release.

- **Volume Control:** With the current moderate storm incidents (1 in 10 year), water can currently flood the roadways, and in a major storm event (1 in 100 year), severe flooding could cause overland flows with the existing system in place. As such, the UBC Integrated Stormwater Management Plans recommend storage capacity able to contain 2,500 to 3,000 cubic meters [1], which our team met and exceeded with a capacity of over 3,400 cubic meters.

- **Erosion and Environmental Protection:** Protection of the geotechnically and environmentally sensitive surrounding cliffs in Point Grey was a vital consideration. Even with the limited flow capacity of 1.2 m3/s, the discharge of the water all the way to the ocean being safe and not deteriorating the sandy cliffs was considered high priority.

- **Social Impact:** The project is in close proximity to extensive green areas and the UBC farm, and as such our design aims to have the aesthetics and use for a smooth integration between the environment and infrastructure which could be desirable for the surrounding community.

- **Multi-Usability:** Being a key design criteria, multi- usability was high priority item for the development of our design, and as such the Velodrome and park design has been engineered to encourage a functional multi-purpose facility and physical activity.

- **Economic Impact:** In order to facilitate UBC’s promotion of greener stormwater management strategies, our team strived to create an affordable project through efficient construction time, labor, and material costs.

- **Design Life:** The design life for this project has been established to be 100 years. This was selected on the basis that our design needed to account for a 1 in 100 year storm, and as such, it was designed to be equipped to withstand that duration.

- **Design Loadings:** The loadings selected for our final detailed design were used to determine the design of the foundation, thickness of concrete slabs, design slopes, and amount of required steel rebar reinforcement.
3.0 DESIGN COMPONENTS

The multi-use storm-water detention system used is divided into separate components to simplify the analysis and design of each individual component. In this regard, the detention system is split into the main velodrome structure, and the rain garden components. The analysis and design of these components is presented below.

3.1 Velodrome structure

The main velodrome structure is broken down into following design components for ease of design: the bicycle tracks, the retaining wall and the footing. These components are discussed individually as follows.

3.1.1 Bicycle Tracks

The bicycle tracks are designed using two different design techniques: as slab-on-grade for straight sections and as simply supported one-way slab for end sections, as shown in Appendix D. NBC 2015 specifications as well as CSA A23.3-14 were used in all design considerations, especially with provisions for concrete exposure to soil and chlorides.

The straight sections of the tracks are designed as slab-on-grade since a relatively mild slope of 17° does not pose any challenges in regards to the stability of the excavation. Designing the straight sections of the tracks as slab on grade is also cost effective and relatively easy to construct compared with the end sections.

Live loads were calculated assuming six bicycle riders side-by-side each weighing 80 Kg. Snow-loads were determined per Cl.4.1.6.2 of NBCC 2015. The relation that specifies the calculations of snow loads is reproduced here to elaborate on the different variables listed within it, since these governed the design of the structure:

\[
S = I_s [S_s (C_b C_w C_s C_a ) + S_r]
\]  

[NBCC 2015 CL.4.1.6.2]

where  
- \( S \rightarrow \) Specified load due to snow and associated rain
- \( I_s \rightarrow \) Importance factor for snow load
- \( S_s \rightarrow 1 \) in 50 year ground snow load
- \( S_r \rightarrow 1 \) in 50 year associated rain
- \( C_\cdot \rightarrow \) wind, slope and accumulation factors

As prescribed by NBCC 2015 - Appendix B, the ground snow load for the Vancouver (Granville & 41 Ave.) was used since that is the area closest to the site of the proposed project. The relevant table of specified loads is shown in Appendix B. Since the structure is not designed as a habitable
structure, and is outdoors, away from residential buildings, the importance factor I_s was chosen as 0.8. This ensures that the structure is not over-designed and remains economical to construct while adhering to the safety limits allowed per NBCC.

Table 1: Load Combinations per NBCC 2015

<table>
<thead>
<tr>
<th>Case</th>
<th>Principal Loads</th>
<th>Companion Loads</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>1.4D</td>
<td>-</td>
</tr>
<tr>
<td>2</td>
<td>(1.25D or 0.9D) + 1.5L</td>
<td>1.0S or 0.4W</td>
</tr>
<tr>
<td>3</td>
<td>(1.25D or 0.9D) + 1.5S</td>
<td>1.0L or 0.4W</td>
</tr>
<tr>
<td>4</td>
<td>(1.25D or 0.9D) + 1.4W</td>
<td>0.5L or 0.5S</td>
</tr>
<tr>
<td>5</td>
<td>1.0D + 1.0E</td>
<td>0.5L + 0.25S</td>
</tr>
</tbody>
</table>

Table 1 above lists the load combinations prescribed by the NBCC. Cases 2 and 3 indicate scenarios where a combination of live loads (cyclists) and snow loads may be combined to determine the factored design loads. The scenario where the maximum snow load and cyclists being on the tracks at the same time is not realistic and unlikely. For this reason, load combination case 3 was chosen without adding the companion live loads. This ensures that the structure is not over-designed and remains economical to construct. Thus, the governing load combination was \(1.25D + 1.5S\) and the factored load for the design of the structure was determined to be 11 kPa. The detailed calculations are attached in Appendix C.

The 30.8 m (x2) straight sections will be cast in place. These sections are inclined at a 17° gradient with the horizontal. The site will need to be excavated to a depth of 1.75 m below existing grade with a slope of 17° to allow for the tracks to be cast. Since the site predominantly consists of sandy soils, as confirmed by the geotechnical reports, and the friction angle of the sandy deposits is assumed to be in the range of 35° - 40° the slope of the excavation does not pose any stability issues. The reason is that any excavation slope smaller than the friction angle of the soil is deemed stable. After compacting the excavation with suitable base material, the straight sections will be cast in place.

Since the slab-on-grade section is exposed to soil, CSA A23.3 prescribes that a larger cover depth of 75 mm be used.

The end sections are inclined at a slope of 47° with the horizontal. Should the end sections be excavated at a 47° slope, additional measures of slope stability would need to be in place for safety of workers and equipment. This would add to the cost of operations, and casting on granular soils
at steep slopes poses additional challenges. An easier option is to excavate the site at the end section at a 1V:2H slope. This means that the angle of excavation will be 27° with the horizontal. Being that this is much below the friction angle of the sand, the slope will be stable.

The retaining wall, which is discussed in the next section, will be cast at the base of the excavation. Consequently, the tracks, designed for the end sections as a simply supported, one-way slab, will be placed on top of the retaining wall.

The part of the slab placed on top of the retaining wall was considered a roller joint in designing the slab, whereas the bottom part resting on the footing was considered pinned. This is a safe assumption to make since the bottom end is prevented from any horizontal movement, while being free to rotate. The top end is not a rigid joint and therefore transfers no moment to the retaining wall.

Design criteria laid out by CSA A23.3 was followed and all relevant checks were made to ensure a safe design. The Direct Procedure outlined by Brzev, Pao [1] to determine the area of tension reinforcement required was used. The following checks were performed for the tracks, which were designed as one-way-slabs:

7.8.1 A minimum area of reinforcement of 0.002Ag shall be provided in each direction.
7.8.2 For exposure conditions where crack control is essential, reinforcement exceeding that required by Clause 7.8.1 shall be provided.
7.8.3 Minimum reinforcement shall not be spaced farther apart than the smaller of five times the slab thickness or 500 mm.
7.8.4 At all sections where it is required, minimum reinforcement shall be developed in tension for its specified yield strength in compliance with Clause 12.

In addition, the maximum tension reinforcement requirement specified in Cl.10.5.2 was also checked to ensure that the steel ratio is below the balanced steel ratio so that any failure is steel controlled.

The load combinations used in the design of the end sections of the tracks was the same as that followed for slab-on-grade for the straight sections. The snow loads governed the design. The main difference in the design of the straight sections and end sections of the tracks was the prescribed concrete cover. Since the slab-on-grade section is exposed to soil, a 75 mm cover is used whereas the end sections need only a 60 mm cover since it is not cast against earth.
In all sections, however, the classification C-XL was used since the structure is expected to be exposed to chlorides owing to runoff from roads and ditches and the structure as a whole will also be exposed to freeze-thaw cycles. The CSA A23.3 [2] defines the C-XL class as follows:

**C-XL:** Structurally reinforced concrete exposed to chlorides or other severe environments with or without freezing and thawing conditions, with higher durability performance expectations than the C-I or A-I classes.

### 3.1.2 Retaining wall

As was mentioned in the previous section, a slope of 47° of the bicycle tracks necessitated using retaining walls. The site predominantly contains sandy soils, and assuming an angle of friction of 30° - 35°, any slope over 30° would be unstable and require special measures to stabilize the slope. In the current design, the site will be excavated at a 1:2 gradient (~26°), and after the retaining wall is cast, the portion behind the retaining wall will be filled and compacted.

The type of retaining wall chosen is a free-standing cantilever wall. To ensure safety, it is checked for the overturning, sliding and bearing capacity failure modes. The respective factors of safety are 9.6, 1.56, and 160. The results are acceptable factors of safety (FOS), all exceeding their minimum requirements (2, 1.5 & 3). Calculations are provided in Appendix C.

A wall thickness of 300 mm was selected for the retaining wall. Design calculations yielded that 200 mm thickness was adequate; however, good practice per CSA A23.3 dictates that for wall heights greater than 4m, a 300 mm thick wall be used.

In the design of the retaining wall, the bottom end was assumed rigid. This resulted in designing for negative moment at the bottom end as well as special considerations for an inflection point. Providing reinforcement for the full length of both the inner and outer faces would be unnecessary and result in cost overruns. Therefore, development lengths were calculated and the bars on the side facing soil can be safely terminated at a distance of 1930 mm from the top of the footing. This takes into account the required minimum length past the inflection point required by CSA A23.3.

### 3.1.3 Footing

The footing was designed as a continuous, combined footing per CSA A23.3 specifications. Per NBCC 2015, an 'allowable bearing capacity' of 150 kPa was used in the design. The site contains predominantly sandy soils with some traces of silt in the upper 3 m. However, since the top silty layers will be excavated, the silty layers were ignored in design calculations.
The concrete cover for the retaining wall and the footing was set as 75 mm since it is cast against earth. This fits the condition set by CSA A23.3 that any structure "cast against and permanently exposed to earth" have a 75 mm cover.

### 3.2 Rain Garden

Construction of the UBC Velodrome will require the removal of pervious forest area, replaced with over 2300 m² of impervious concrete. This eliminates the ability for water to infiltrate into the soil and instead adds to the burden on the Westbrook main. The solution is the rain garden center piece of the UBC Velodrome. The rain garden will not only serve as a decorative feature but also a rainwater capture and filtration tool.

It is designed to be a down sloping area with plants to add to the overall aesthetics of the velodrome facility. The rain garden will be 12 meters wide and 24 meters long with a 6 meter curved radius on both ends. The dimensions will result in a surface area of 401 m² and be capable of capturing all the runoff from the velodrome surface for a 6-month 24-hour rainfall event, which accounts for 90% of the storms in the Vancouver area. The design overcompensated for future influxes in storm intensities as the capture capacity of the rain garden approaches that of a 5 year event. Any storm larger than that will result in water reaching the perforated pipe at the bottom of the garden and working its way back to the Westbrook main.

The rain garden will feature a top layer of bioretention soil which allows water to permeate easily while capturing 90% of nutrients and chemicals as well as 80% of sediments. The water will be much cleaner than typical rainwater runoff once it reach the perforated pipe. Under the bioretention soil will be a rock trench where the perforated pipe will run. The rock allows the water to trickle freely to the pipe without getting backed up. A main summary of the rain garden components are shown in Table 2 and the design calculations can be found in Appendix C.

<table>
<thead>
<tr>
<th>Paved Area (m²)</th>
<th>2305</th>
</tr>
</thead>
<tbody>
<tr>
<td>Rain Garden Area (m²)</td>
<td>401</td>
</tr>
<tr>
<td>Topsoil Depth (m)</td>
<td>0.6</td>
</tr>
<tr>
<td>Rock Trench Depth</td>
<td>0.8</td>
</tr>
<tr>
<td>Input Volume (m³)</td>
<td>112</td>
</tr>
</tbody>
</table>
Table 2: Rain Garden Components

Inflow/Outflow:

A diversion pipe will be connected to the existing Westbrook storm main and connect to the UBC Velodrome site to allow for the flow of stormwater into the velodrome detention area during severe storm events. The diversion system is designed to allow normal storm events to continue on their usual paths while only severe flows will be diverted. The weir controlled system can be seen below in Figure 1.

![Weir Controlled System (City of Sacramento, 2000)](image)

As the inflow pipe approaches the velodrome detention area it will start to fan out to disperse the energy of the flow. The water will be released into the velodrome via a long thin opening near the top of the northernmost straightaway. The inflow opening will be 8 meters long by 0.3 meter high. Images of the inflow opening and how the velodrome will fill with water can be seen in Appendix E. Cyclists do not ride near the top of the straightaways, so the inflow opening will not interfere with riders.

The rock pit under the rain garden will have a 0.45 meter perforated pipe in the middle, laid at a 2.0% slope towards Westbrook. The pipe will take effluent water away from the site at a controlled rate of around 0.40 m³/s. The discharge rate is well under the 1.2 m³/s maximum recommended by the UBC Integrated Stormwater Management Plan.
For storms larger than what the rain garden is designed to handle, two elevated catch basins will be in the rain garden to transport water directly to the outflow pipe. The overflow catch basin will be 0.5 meters above the surface of the rain garden at the center level.

### 3.3 Concrete Mix Design & Concrete Specifications

The concrete mix design for this structure will be coordinated between the contractor and the concrete supplier to meet the requirements outlined in Table 3 below which outline the requirements specified in CSA A23.1 and CSA A23.2 for concrete Mix Proportions. Some of the parameters that should be noted are:

- Maximum aggregate size for the mix design will be 30 mm.
- Air content for the mix will be 4-7% for the hardened concrete.
- The specified 28-day strength of the concrete will be 25 MPa.
- The exposure class is C-1, structurally reinforced concrete exposed to chlorides with or without freezing and thawing conditions (Cl.4.1.1).
- The concrete shall be designed to have a finished density of approximately $2400 \text{ kg/m}^3$ (Cl. 4.3.4.1).

<table>
<thead>
<tr>
<th>Requirement (a)</th>
<th>4.3.2.2 Nominal maximum size of aggregate MSA less than or equal to (a)-(d)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Workable concrete [Clause 4.3.2]</td>
<td>(a) ⅕ of the narrowest dimension between side forms = 60mm</td>
</tr>
<tr>
<td></td>
<td>(b) ¾ of minimum clear spacing between rebars = 214mm</td>
</tr>
<tr>
<td></td>
<td>(c) ⅔ of the depth of the slabs = 100mm</td>
</tr>
<tr>
<td></td>
<td>(d) The specified concrete cover not exposed to earth or weather = 60mm</td>
</tr>
<tr>
<td></td>
<td>(e) ⅔ of the specified cover for concrete exposed to earth or weather = 50mm</td>
</tr>
<tr>
<td></td>
<td>(f) ⅓ of the specified cover for concrete exposed to chlorides = 30mm MSA [governs]</td>
</tr>
</tbody>
</table>

4.3.2.3 Slump or Slump Flow

Slump requirements shall be identified and reviewed by the contractor and concrete supplier prior to construction. When the slump is specified, the acceptance of the concrete in the field shall be subject to the tolerances specified in Clause 4.3.3.2.
| Requirement (b) | All of clauses 4.1.1.1 to 4.1.1.10 should be adhered to. Placing of concrete will be conducted by the contractor in accordance with clause 7.4: Placing of Concrete, to ensure durability requirements are met. | **Concrete Durability**  
[Clause 4.1.1]**

**Table 1 Classes of exposure**
C-1 - Structurally reinforced concrete exposed to chlorides with or without freezing and thawing conditions.

**Table 2 requirements [C-1]**
- Maximum water-to-cementing materials ratio = 0.4
- Air content category as per table 4 = 4-7% for hardened concrete and 3-6% at the point of discharge from delivery equipment.
- Curing type as per table 19, additional curing (7 days >10 degrees celsius should be provided).

| Requirement (c) | As per table 4  
For freshly delivered concrete: AC = 3-6%  
For hardened concrete: AC = 4-7% | **Air Content**  
[Clause 4.3.3]**

| Requirement (d) | 4.3.4.1 Normal-density concrete  
The concrete used for this structure will be Normal Density concrete which will be measured in accordance with CSA A23.2-6C for fresh concrete. | **Required density**  
[Clause 4.3.4]**

| Requirement (e) | 4.3.5 Strength  
The 28 day strength of the concrete must be greater than or equal to 25MPa. | **Specified Strength**  
[Clause 4.3.5]**

| Requirement (f) | 4.3.6 Volume stability considerations  
Creep and drying shrinkage can be minimized by:  
(a) Maximizing the nominal aggregate size = 30mm  
(b) Minimizing water content  
(c) Has a grading of fine and coarse aggregates that allow for the required workability while minimizing water content. | **Volume Stability**  
[Clause 4.3.6]**

| Requirement (g) | 4.2.3.6 Deleterious reactions of aggregates  
Considering the nature of the structure, only aggregate sources that are confirmed to be clear of alkali materials and other reactive materials that could crack the concrete and make this structure unfit for service. | **Prevent Concrete Expansion**  
[Clause 4.2.3.6]**
Table 3: Concrete Mix Proportions (CSA A23.1):

3.4 Concrete formwork

The simplicity of the concrete forms permits the contractor to select one of their common methods for constructing formwork for a retaining wall and footing system in accordance with CSA A23.1 Clause 6.5. The fabrication and erection of the formwork must be approved by the Engineer of Record and constructed in accordance with CSA-S269.3.

The formwork selected by the contractor shall be determined by the dimensions outlined in the drawings in Appendix D, D.1.
4.0 CONSTRUCTION METHOD

4.1 Construction Requirements

In accordance with the contractual terms of the project, our team will undertake the construction of the project as per the specified design. In doing so, certain requirements are to be met in the construction of the UBC Velodrome. Listed below are key considerations regarding general construction requirements. These requirements may pertain to not only the constructing party (our team), but the owners of the project (UBC):

- Work on site shall abide by our team’s approved Health and Safety (H&S) work guidelines. These guidelines are to abide by WorkSafe BC’s general guidelines and may include:
  - Company specific daily work sign-offs;
  - Machine pre-use inspection checklists;
  - Proper confined space operations;
  - Work requiring certifications with procedures or machinery will require verification of certifications (fall protection, due diligence, crane, etc);
  - A zero drug or alcohol policy on site.

- Material used for the construction of the UBC Velodrome must abide by Canadian / design guidelines. The conformity of these materials shall be checked on site and logged with a Material Reception Inspection (MRI) sheet;

- Work processes are to abide by either contractual terms or general industry practices. The work shall be undertaken with due diligence and any practice not in compliance with these standards shall be terminated and the practicing party will face liability with regards to the work;

- Subcontractors undertaking jobs shall conform to the same work and H&S policies as the sub-contracting party;

- Change orders must first be specified and approved upon method and cost before work can be undertaken;

- Certain work or processes shall be checked at interval or completion to ensure conformity (concrete tests, soil compaction tests, leak tests, etc).

4.2 Work Breakdown Structure

In the construction of the UBC Velodrome, milestones care attributed to the completion at each major tasks or part. These tasks are defined by its governance over smaller tasks and its independency to ensuing tasks. As such, these tasks are also use as a measure in the completeness
of the project which in turn is used as a benchmark for contractual payments. The complete list of tasks can be seen in the work breakdown structure below.

<table>
<thead>
<tr>
<th>Phase of Project</th>
<th>Task Number</th>
<th>Description</th>
<th>Additional Classification</th>
</tr>
</thead>
<tbody>
<tr>
<td>1.0 Preconstruction</td>
<td>1-001</td>
<td>Environmental Assessment (EA) collection of information, submissions and actions.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>1-002</td>
<td>Public Notifications and Consultation meetings.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>1-003</td>
<td>Follow up with EA to receive permitting,</td>
<td></td>
</tr>
<tr>
<td></td>
<td>1-004</td>
<td>Geotechnical Consulting</td>
<td></td>
</tr>
<tr>
<td></td>
<td>1-005</td>
<td>Environmental Consulting (Work by Biologists applies here as well)</td>
<td></td>
</tr>
<tr>
<td></td>
<td>1-006</td>
<td>Original Ground (OG survey) tied up with survey control in place. Survey control to be used for construction survey and as-built.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>1-007</td>
<td>Measurements, marking and staking for first phases of construction.</td>
<td></td>
</tr>
<tr>
<td>2.0 Construction</td>
<td>2-001</td>
<td>Removal of trees as required for access and construction.</td>
<td>Site Preparation &amp; Earthworks</td>
</tr>
<tr>
<td></td>
<td>2-002</td>
<td>Access path construction from Westbrook Mall, entrance and exit to site.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2-003</td>
<td>Procurement of materials. (note: certain materials will require weeks or even months of advanced notice before they can be delivered).</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2-004</td>
<td>Area for onsite office, muster point and outhouse facilities established. Storage areas for new aggregates, crush, soils established. Storage areas for waste materials established.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2-005</td>
<td>Excavation earthworks for upstream piping, the containment area, and downstream piping.</td>
<td></td>
</tr>
<tr>
<td></td>
<td>2-006</td>
<td>Engineering controls for slope stability during construction installed (e.g.) sheet piles. Engineered controls for drainage during construction installed (e.g.)</td>
<td></td>
</tr>
<tr>
<td>2.0 Construction</td>
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<td>2-007 pump and hose system.</td>
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<td>2-008 Placement of material for subgrade placed and compacted. Specifications for placement and compaction will be provided for the pipe trenches and the Velodrome location.</td>
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<td>2-009 Backfill and compaction for pipe trenches.</td>
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<td>2-016 Formwork: Straight Segments of Velodrome</td>
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<td>2-032</td>
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<td>Formwork: Periphery of the Velodrome</td>
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<tr>
<td>2-038</td>
<td>Fill in any minor imperfections as directed by the Engineers representative on site.</td>
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<td>2-039</td>
<td>Seal coating Concrete</td>
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<td>2-040</td>
<td>Attach influent pipe to inlet at the North East side of the Velodrome. Attach the effluent pipe to the outlet of the Velodrome at the base in the middle of the structure.</td>
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<tr>
<td>2-041</td>
<td>In combination with task 2-039, construct the designed water quality systems including: a large debris and small debris filter.</td>
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<td>2-042</td>
<td>Install shut off valves and diversion valves at specified locations.</td>
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<tr>
<td>2-043</td>
<td>Complete the infilling of granular fill and soils around the periphery of the Velodrome, compact the fill and soils to the specified standard proctor density and up to the elevation specified in the plans.</td>
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<tr>
<td>2-044</td>
<td>Place permeable asphalt in the middle of the velodrome, surrounding the outlet.</td>
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<tr>
<td>2-045</td>
<td>Place top soils between the concrete track and the permeable asphalt. Grade the soils and seed the surface as specified in the plan drawings</td>
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<th>3.0 Site Finishing &amp; Demobilization</th>
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<tr>
<td>3-001</td>
<td>Landscape the turf area inside the Velodrome and the area outside of the Velodrome according to the landscape architecture plans.</td>
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<tr>
<td>3-002</td>
<td>Install and construct the specified benches, decorative structures, curbs, bike racks, and the permanent entrance sign and post board.</td>
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<tr>
<td>3-003</td>
<td>Remove debris piles from site and ensure that any excess stockpile material is accounted for and stockpiled in a different location.</td>
</tr>
<tr>
<td>3-004</td>
<td>Deconstruct and remove the site office, outhouse facilities and any other temporary structures.</td>
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</table>

**Table 4: Work Breakdown Structure**
4.3 Construction Schedule

In the scheduling of work for the UBC Velodrome, several factors are considered when developing the timeline of work. First, it is expected that the turnaround for reports, requests for information (RFIs), submittals and approvals to be a maximum of 14 days. This value is obtained from general Metro Vancouver construction standards and practices. Secondly, the scheduled timeline assumes that work will be done with a crew of 5 laborers overseen by 1 foreman, 1 superintendent, and 1 project coordinator. However, this is subjected to change for activities that requires heavy man-hours and that are on the critical path. An example of this would be the slab formwork, rebar, and pour of the side segments of the velodrome: given that the this slab pour shall include the piping system for the filtered water removal (which is needed due to the high amount of debris expected in a 100 year return period storm), a crew of 10 is anticipated in such instance. Furthermore, items such as landscaping and piping install may be subcontracted out to other companies. However, the timeline shall remain the same in the instances that it does. With regards to man-hours, the values are accurate and reflect RSMeans standards for construction work. Lastly, the schedule takes into account all statutory holidays as well as a two week Christmas break. As the project is set to begin on April 29th, 2019 and finished June 12th, 2020 with a total of 276 working days. The complete schedule can be seen below; a full size version can also be found in the appendices.

![UBC Velodrome Construction Schedule Gantt Chart](image)

4.4 Quality Assurance

The on-site work, both done by our crew members and through our subcontractors, are to be overseen by our team. As previously mentioned in 4.1 Construction Requirements, site work will
follow certain guidelines and are subjected to inspection by not only our team’s engineers, but UBC’s as well. Summarized below are recurring inspections to ensure the adequacy of the project.

- Bi-weekly Environmental Assessments (EA) reports are to be conducted with respect to site cleanliness, quality of water runoffs, debris, and trailing;
- Compaction tests are to be completed and logged following soil placements to ensure design compactions (90% compaction on untraveled walkways, 95% on traveled walkways);
- Concrete tests are to be conducted for each batch of concrete poured to ensure that they meet the design strength;
- Rebar ties and formwork are to be inspected by a structural engineer to ensure its conformity;
- Leakage and drain rates shall be checked by a Building Science Consultancy to ensure standards are met;
- Water run offs are to be check and approved prior to the commissioning of the UBC Velodrome to ensure that standards are met.

### 4.5 Anticipated Issues

There are several anticipated on-site issues regarding the logistics of this project. These issues pertain to either an environmental, weather, by-law or geographical consideration. They are expanded upon more below with actions our team plan to take in addressing them.

#### 4.5.1 Traffic Interruptions

Given that the construction site exists adjacent to two-lane Wesbrook Mall, it can be observed that there will be significant traffic interruptions during the period of construction. This is because Wesbrook Mall acts as the main connector between the Wesbrook Village community and South West (SW) Marine Drive, which is one of the major roads that connects UBC to the rest of Vancouver, Richmond, and Surrey. Furthermore, the traffic interruptions will be especially disruptive during school seasons give that Wesbrook Mall is a connecting road for various bus routes entering UBC. Another thing to consider is that the traffic of construction vehicles entering the job site will be especially high during concrete pours. With a total volume of 830 cubic meters of concrete that will need to be poured and the average concrete truck carrying 10 cubic meters during the period of August 20th, 2019 and January 30th, 2020, it can be seen that there will be at least 83 visits (and thus, instances of disruption) between that period.

To address these issues, a traffic permit must first be applied for before construction can proceed. Then, adequate steps will need to be taken to warn stakeholders of the area of the impending
disruptions. This can be done through stakeholder meetings, mail updates, and road signs warning of the impending construction. Steps will also be taken by our team to manage the traffic when total lane closures are required. Lastly, our team will look to consult Translink on whether there is a need for changes to the bus schedules and routes for any buses passing through the area.

4.5.2 Winter Productivity

Given the unpredictable circumstances of Vancouver climate over the past few years, there may lie a possibility for weather to be a detrimental factor in the construction of the UBC Velodrome. It is understood that unfavourable weather conditions impacts the productivity of workers from both a psychological and physiological standpoint (Ibbs & Sun, 2007). In addition to this, however, there may lie a possibility for weather to interfere with the concrete pours of the project. Given that concrete pours are to take place largely in the Winter months (August 20th, 2019 to January 30th, 2020), extreme weather conditions may make it such that extra precautions will need to be undertaken to ensure that the concrete pours will proceed without hiccup. These precautions may include adequate concrete cover after pours or introducing methods to heat the concrete such that the concrete will cure properly. However, this is already reflected in the scheduling of the project. The development of the concrete pours (rebar, formwork, etc) are assigned a 50% time contingency to allow for disruptions should as these. However, this is also be an indicator that the project can finish ahead of schedule given favourable conditions. In cases of extreme weather such as a snow storm, it can be observed that work may have to stop altogether.

4.5.3 Specialized Design

Given that there have been only 12 velodromes ever constructed in Canada, two of which reside in British Columbia, it can be understood that there may be a learning curve associated with its construction and design. In addition to that, the UBC Velodrome will be the first of its kind in Canada wherein it serves as a multi-use water detention facility for a 100- year return period storm. Thus, we have reached out to prior constructors and consultancies of Velodromes and water retention systems for advice and our team plans to retain a consultancy specializing in hydro-retention to oversee our force calculations.
5.0 COST ESTIMATE

Given the development stage of this project, this report includes an updated Class B Cost Estimate as per EGBC’s Classification Definition:

“Class B estimate (estimate (±15-25%): An estimate prepared after site investigations and studies have been completed and the major systems defined. It is based on a project brief and preliminary design. It is used for obtaining effective project approval and for budgetary control.” (EGBC, 2009).

Our Cost Estimate depicting work to be performed has the following categories: environmental and permitting, civil, mobilization, man hours, site finishing and demobilization, and annual operating and maintenance. Contingency is applied as per the EGBC Class B definition above, using the upper limit of 25% due to the size and potential for unknowns in the project. Further, project design and construction management fees are included as 15% of the total cost. The detailed Class B Estimate is as shown below.

Some key features in this estimate have been able to be updated due to the completion of the detailed design and calculations, enabling a higher level of accuracy and precision. Some of these include the following. Concrete and rebar quantities were increased as per RSMeans recommendations. Due to the over year-long duration of the project, the potential need for ongoing smaller environmental assessments in addition to one large one at the end of the project was identified. Additionally, more testing of products were accounted for, including concrete strength testing for each truckload and concrete leak testing upon placement and curing. As the scheduling has been updated, the man hours have been as well in accordance, and a site surveyor and project coordinator were added to the crew. The cost of site cleanup was increased again to acknowledge the duration of the project, and that site clean up may need to occur routinely, rather than just once at the end of the project.

The largest change to this estimate was the finalization of service life operating and maintenance plan. The design life of the project has been established to be 100 years as per the 1 in 100 year storm event, which enabled durations to be applied to our existing annual operating and maintenance costs. Further needs that were identified and addressed were annual concrete seal coating, concrete resurfacing every 3 years to ensure rider safety, and an allocation for the clean up and landscaping of a 1 in 100 year storm event. A key feature of the service life operation and maintenance costs section is that economic analysis was performed to bring the costs over the duration of the 100 year project to present worth costs. The current interest rate was taken to be
1.75% which was found from the Bank of Canada database. The rest of the costs were considered to be within an immediate enough time period (i.e. take place over approximately the next 1 year) so as to be reasonably already in present worth.

### Class B Cost Estimate

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<th>Quantity</th>
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6.0 CONCLUSION

This facility will be an excellent way of providing a stormwater detention site, which will alleviate the risks of 10 and 100 year rainfall events, while serving a dual purpose as a public Velodrome which is centred by a Rain Garden. The total capacity of the detention facility is over 3,400 cubic metres which allows the discharge of water to the ocean to be below the maximum of 1.2 cu.m/s. The project will begin April 29th, 2019 and will be concluded on June 12th, 2020, totalling 276 work days. Finally, the all-included cost of the project is estimated at $12,106,026 and is designed for a 100-year lifespan.

References


# APPENDIX A: CONSTRUCTION SCHEDULE

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APPENDIX B: STANDARDS AND CODES

4.1.6.2. Specified Snow Load

(See Note A-4.1.6.2.)

1) The specified load, \( S \), due to snow and associated rain accumulation on a roof or any other building surface subject to snow accumulation shall be calculated using the formula:

\[
S = I_s [S_r (C_s C_w C_a C_e) + S_f]
\]

where

- \( I_s \) = importance factor for snow load as provided in Table 4.1.6.2-A.
- \( S_r \) = 1-in-50-year ground snow load, in kPa, determined in accordance with Subsection 1.1.3.

Table 4.1.6.2-A

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<th>Importance Category</th>
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* This refers to the vapour barrier.
† The cover for aggregate is.
### Table C-2 (Continued)

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<th>Ann. Rain, mm</th>
<th>Most Index</th>
<th>Ann. Tot. Pn, mm</th>
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</table>
UBC Velodrome Rain Garden Design Calculations
(Calcs based on Kerr Wood Leidal Ltd. Rain Garden Design Considerations)

Input Volume

<table>
<thead>
<tr>
<th>Paved Area (sm)</th>
<th>2305</th>
</tr>
</thead>
<tbody>
<tr>
<td>2 yr, 24 hr (mm/hr)</td>
<td>2.8</td>
</tr>
<tr>
<td>(metrovancouver.org, Estimated 2050)</td>
<td></td>
</tr>
<tr>
<td>Period (hrs)</td>
<td>24</td>
</tr>
</tbody>
</table>

2-yr Rainfall Event

Rainfall Intensity x Period

67 mm

6-month Rainfall Event (90% of storms in area)

2-yr Rainfall event x 72%

48 mm

Input Volume:

Paved Area x 6-month Rainfall Event

112 cm

Capture Volume:

<table>
<thead>
<tr>
<th>Rain Garden Area (sm)</th>
<th>401</th>
</tr>
</thead>
<tbody>
<tr>
<td>Evaporation Rate (mm/day)</td>
<td>1</td>
</tr>
<tr>
<td>(Avg for Metro Van. In winter)</td>
<td></td>
</tr>
<tr>
<td>Topsoil Depth (m)</td>
<td>0.6</td>
</tr>
<tr>
<td>(Bioretention Soils)</td>
<td></td>
</tr>
<tr>
<td>Field Capacity</td>
<td>0.25</td>
</tr>
<tr>
<td>Wilting Point - Sandy Soils</td>
<td>0.05</td>
</tr>
<tr>
<td>Typical Trench Depth (m)</td>
<td>0.8</td>
</tr>
<tr>
<td>Voids Ratio - Sandy Soils</td>
<td>0.35</td>
</tr>
<tr>
<td>(Range 0.3-0.5)</td>
<td></td>
</tr>
<tr>
<td>Infiltration Rate (mm/hr)</td>
<td>1.5</td>
</tr>
<tr>
<td>(Glacial Till)</td>
<td></td>
</tr>
</tbody>
</table>

Evaporation Vol. = Rain Garden Area x Evaporation Rate

0.4 cm

Growing Medium Vol. =

Rain Garden Area x Porosity x (Field Capacity - Wilting Point)

48.1 cm

Rock Trench Vol. = RG Area x Trench Depth x Voids Raio

112.3 cm

Infiltration Rate = Rain Garden Area x Infiltration Rate

14.4 cm

**TOTAL CAPTURE VOLUME** 175.2 CM

100 % Capture Rate
**Volume Calculations - UBC Velodrome**

**Total Volume** = \( A + 2B + 2C + 2D \)

**Constraint:** \( H_{\text{max}} = 1.75 \text{ m} \)

**A**

\[ W \times H \times L = 30.8 \text{ m}(1.75 \text{ m})(27.52 \text{ m}) \]

\[ A = 1483.33 \text{ m}^3 \]

**B**

\[ \pi r^2 H = \pi (16.5)^2 \times 1.75 \text{ m} \]

\[ B = 1496.77 \text{ m}^3 \]

**C**

\[ L \times W \times H = 30.8 \text{ m} \times 5.74 \times 1.75 \]

\[ C = 309.39 \text{ m}^3 \]

**D**

\[ \tan 47^\circ = \frac{1.75}{x} \]

\[ x = 1.63 \text{ m} \]

\[ r_1 = 16.5 \quad r_2 = 18.13 \]

\[ \pi (r_2^2 - r_1^2) \times H = \pi (18.13^2 - 16.5^2) \times 1.75 \]

\[ D = 310.33 \text{ m}^3 \]

\[ 3599.8 \text{ m}^3 \]

\[ \rightarrow \sim 3600 \text{ m}^3 \]
The average soil pressure on the wall:

\[ p = \frac{1}{2} (f_a + f_r) \left( \frac{H}{D} + H \right) \]

\[ p = \frac{1}{2} (1.2) \left( \frac{15.2}{2.5} + 15.2 \right) \]

\[ p = \frac{1}{2} (1.2) (10) = 6 \text{ kPa} \]

The soil pressure needs to be specified per NBS 2015.

Example:

1. \( f_a = 1.5 \) kPa
2. \( f_r = 1.5 (10 + 5) \) kPa

\[ M = \frac{p H^2}{8} = \frac{(15.2 \text{ kPa})(4.6)}{8} = 40.2 \text{ kNm} \]

Combined gravity and lateral loads.

A wall for drainage is designed by superimposing the

Resultant. Resultant she wall. Forces result

given and soil. Vertical loads:

\[ F_{vert} = F_{hor} = 24.3 \text{ kN} \]

Mark of zero moment:

The moment at this point:

\[ M = \frac{F_{vert} L}{2} = \frac{24.3 \times 4.6}{2} = 54.6 \text{ kNm} \]

The moment diagram due to horizontal gravity and lateral. Loads is as follows:

The moment at the top:

\[ M_{top} = 20.1 \text{ kNm} \]

The moment at the bottom:

\[ M_{bottom} = 54.6 \text{ kNm} \]
Design the flexural reinforcement for the footing and exterior wall face.

For the footing, use 20 M14 bars.

**Effective Depth - Footing**

- **Cover = 75 mm**
- **433.3 - Outdoors, submerged in earth**
- \( t = \frac{400}{0.8} \) mm
- \( d = t - \text{cover} - \frac{d_o}{2} \)
- \( = 380 - 75 - \frac{20}{2} \) mm

\( d = 235 \text{ mm} \) (effective)

**Effective Depth - Wall**

- **Cover = 75 mm**
- **433.3 - Outdoors, exposed to earth**
- **MIN. WALL THICKNESS**
- **MAX. IF**

**Minimum Limit**

\( d = \frac{t}{2} - \text{cover} - \frac{d_o}{2} \)

\( = \frac{380}{2} - 75 - \frac{20}{2} \) mm

\( d = 215 \text{ mm} \)
The required moment at the centroid:

\[ M_y = M_x = 47.3 \text{ kN-m} \]

Find the required area of vertical tension steel:

Choose the area of steel required based on a span of 2000 mm.

Using direct method:

\[ A_y = 0.0015 \frac{d}{N} \left( \frac{d - 0.35a}{a} \right) \]

\[ = 0.0015 \left( \frac{1000}{100} \right) \left( \frac{200 - 0.35(43.3 \times 10^{-6})}{200} \right) \]

\[ A_y = 67.3 \text{ mm}^2 \]

Using 20.44 mm diameter:

Spacing 5 & 4.4, 1000

\[ A_y = 67.3 \text{ mm}^2 \]

\[ 5 & 4.4, 1000 \]

\[ 5 & 4.4, 1000 \]

\[ 67.3 \text{ mm}^2 \]

\[ 67.3 \text{ mm}^2 \]

\[ 5 & 4.4, 1000 \]

Max. Admitted Bar Spacing, \( d \) (19.1, 8.9)

\[ d_{max} \leq \sqrt{\frac{31}{500}} \]

\[ d_{max} = 900 \text{ mm} \]

Since \( 500 \text{ mm} < 900 \text{ mm} \), it is ok!

Theoretical Vertical Reinforcement: 20 bars @ 4.4

Total Needed Reinforcement: 20 bars @ 4.4

Longitudinal reinforcement, the CA 4.33 mm square, has been satisfied.

Actual Reinforcement:

\[ A_y = \frac{1080 \times 1080}{5} = 1200 \text{ mm}^2 \]

\[ A_y = 750 \text{ mm}^2 \]

Check that tensile is still controlled:

\[ f_y A_y = 750 \]

\[ f_y = \frac{750}{1080 \times 2080} = 0.023 \]

\[ f_y < f_y = 0.023 \]

fy = 0.023
CHECK THE CSA MINIMUM NAIL ASSEMBLY

\[ A_y = 100 \, \text{mm} \times 1 \]
\[ A_y = 100 \, \text{mm} \times 300 \, \text{mm} \]
\[ A_y = 30,000 \, \text{mm}^2 \]

\[ A_{n,m} = 0.0015 \times A_y \]
\[ A_{n,m} = 0.0015 \left( \frac{30,000}{10,000} \right) \]
\[ A_{n,m} = 450 \, \text{mm}^2 \]

Since \( A_{n,m} = 450 \, \text{mm}^2 \geq A_{n,m} = 450 \, \text{mm}^2 \), it is acceptable.

Determine the length of the 20-HS bar above the footing (The development 400-mm):

\[ f = 0.45 (2) (1) (1) (0.8) \frac{20}{0.8} \frac{90}{10} \frac{1}{0.8} \]
\[ f = 976 \, \text{mm} \]

CH 12.11.3 Should one specify the full bar length?

\[ l_{d} < \frac{l_{y}}{f_{y}} \]

\[ l_{d} \leq l_{d} \]

\[ l_{d} \leq l_{d} \]

\[ l_{d} \leq l_{d} \]

Therefore, the total wall reinforcement length above the wall footing interface can be determined as:

\[ l = 1.35 + 0.576 \, \text{m} \]
\[ l = 1.93 \, \text{m} \]
SLAB/RETAINING WALL/FOOTING DESIGN

1) Design the vertical and horizontal reinforcement at the interior wall face.

- Design the wall for internal forces:
  - Cover = 60 mm
  - Distance to column:
    - $d = 700$ mm
  - $c = \frac{h - c_v}{2}$
  - $d = 300 - 60 - 20 = 220$ mm
  - $d = 220$ mm

- Find the required moment resistance:
  - $M_v = M_y = 220$ kN.m

- Find the required area of vertical tension reinforcement:
  - Design the wall for tension based on a strip of unit width $n = 1000$ mm
  - Using direct method:
    \[
    A_y = 0.0045 \frac{h}{d} \left( \frac{d}{h} \right)^2 \sin \theta \left( \frac{d}{h} \right) \left( \frac{h}{d} \right) \left( \frac{d}{h} \right)
    \]
    \[
    = 0.0045 \left( \frac{300}{220} \right) \left( \frac{220}{300} \right) \left( \frac{300}{220} \right) \left( \frac{220}{300} \right)
    \]
    \[
    A_y = 241.5 \text{ mm}^2
    \]
  - Masonry is 500 mm thick. Use 15 M bars. ($A_y = 241.5 \text{ mm}^2$)

2) Find maximum permitted bar spacing for $d = 3:1.5:9$

- Space $\leq \frac{3d - 3 \times 300}{500} = 900$ mm

- Since $S_{max} = 900 \leq S = 750$

- Use $b = 500$ mm

- Vertical reinforcement is $15M(500)$

- Check that the minimum area requirement is satisfied:
  - $A_y = \frac{A_b}{s} \cdot \frac{500}{300}$
  - $A_y = 400 \text{ mm}^2$

- $P = \frac{A_y}{500} \cdot \frac{400}{300}$

- $P = 0.0074$

- Since $P < P_0 = 0.022$, proceed.

- Check that the minimum area requirement is satisfied:
  - $A_y = 1000 \times \frac{400}{300}$
  - $A_y = 400 \text{ mm}^2$

- $A_y = 0.0015 A_y = 0.0015 \left( \frac{300}{220} \right)$

- $A_y = 490 \text{ mm}^2 < A_y = 800 \text{ mm}^2$
INCREASE THE AREA OF STEEL TIES:

\[
\frac{A_g}{A_{min}} = \frac{964}{950} = 1.02. \]

\[
A_g = A_{1000} = 200 \times 1000 = 200000 \text{ mm}^2.
\]

\[
A_{1000} = \frac{964 \times 1000}{200} = 482 \text{ mm}^2.
\]

\[
A_g > A_{1000} = 482 \text{ mm}^2.
\]

CHECK MAX A axes requirement:

\[
\frac{P}{A_g} = \frac{964}{200} = 4.82 \text{ kN/mm}. \]

\[
\frac{P}{A_{1000}} = \frac{964}{482} = 2.00 \text{ kN/mm}. \]

\[
P > 2A_{1000} > P > A_{1000} \text{ kN/mm}. \]

DESIGN THE WALL FOR SHEAR:

DESIGN CHECK FORCE:

\[
V_f = 45.3 \text{ kN}. \]

CHECKING \( V_f \) THE WORKABLE SHEAR RESISTANCE:

\[
V_f = \frac{964}{600} = 1.61 \text{ kN/mm}. \]

\[
\frac{964}{964} = \frac{324}{324} = 1.23 \text{ kN/mm}. \]

\[
\frac{216}{216} = 1 \text{ kN/mm}. \]

\[
\therefore \Delta = 2.16 \text{ mm}. \]

\[
\beta = \frac{230}{1000 + \Delta} = \frac{230}{1021} = 0.223 \text{ kN}. \]

\[
\therefore \frac{V_f}{V_f} = \frac{964}{125} = 0.77 \text{ kN}. \]

\[
V_f = 125.8 \text{ kN}. \]

\[
\therefore \Delta = 94.3 \text{ kN} < V_f = 125.8 \text{ kN} \therefore \text{OK!}
\]

AND SHEAR REINFORCEMENT IS NOT NECESSARY.

However, 423.3 kN requirements adequate minimum reinforcement for horizontal reinforcement in wall.

Determine the maximum distributed force along:

1) ALIGNMENT AXES:

\[
A_g = 1000 \times x_t - 1000 \times 100 = 300 \times 100 = 30000 \text{ mm}^2.
\]

\[
A_{1000} = \frac{964 \times 1000}{300} = 3.21 \text{ mm}^2.
\]

\[
A_g = 600 \text{ mm}^2.
\]

2) DETERMINE THE PERM. BAR STANCING:

USING 15 M BARS:

\[
5 \times A_g = 5 \times 250 = 1250 \text{ kN} \times \frac{15}{5000} \text{ mm}. \]
Check whether the covering is within limits prescribed by AS33 198.4

\[
\begin{align*}
S_{min} & \geq 3 \times 300 - 900 \text{ mm} \\
& \geq 300 \text{ mm}
\end{align*}
\]

Check \( S - 300 \text{ mm} < S_{min} = 900 \text{ mm} \) : ok!

Use [150 x 200] transverse reinforcement.
80 kN x 6 = 480 kN, 

\[ = 480 \times 0.8 \]

\[ = 470.4 \sim 4710 \text{ N} \]

\[ = 470.4 \text{ kN} \times 1 \]

\[ \text{Snow Load} \]

\[ S = 2 \left( S_x + \left( S_y + S_z + S_u \right) \right) \]

\[ S_x = 0.8 \]

\[ S_y = 1.9 \text{ kN/m} \]

\[ S_z = 0.3 \text{ kN/m} \]

\[ S_u = 0.8 \times 1.9 + 0.3 \]

\[ S = 1.82 \text{ kN/m} \]

\[ = 1.82 \text{ kN/m} \times 1 \]

\[ S = 1.82 \text{ kN/m} \]

\[ \text{Dead Load} \]

\[ = 420 \times 1.5 \text{ kN/m} \times 3 \text{ m} \times 1 \text{ m} \]

\[ \text{For a 1m x 3m x 0.3m thick wall, } \]

\[ D = 972 \text{ kN/m} \]

\[ \text{Load Combinations} \]

\[ 1.4D \rightarrow 1.4 \times \left( 972 \text{ kN/m} \right) = 1365 \text{ kN/m} \]

\[ 1.25D + 1.0S \rightarrow 1.25 \times \left( 972 \text{ kN/m} \right) + 1.0 \times \left( 182 \text{ kN/m} \right) \]

\[ = 1159 \text{ kN/m} \]

\[ 1.25D + 1.5L \rightarrow 1.25 \times \left( 972 \text{ kN/m} \right) + 1.5 \times \left( 0.8 \text{ kN/m} \right) \]

\[ = 1231 \text{ kN/m} \]

\[ \text{Design}\] 

\[ d = 110 \text{ mm} \]

\[ A_0 = 0.0015 \times 110 \left( 300 \text{ mm} \right) = 55 \text{ mm} \]

\[ A_s = 0.0015 \times (1000 \text{ mm}) \times (25 \text{ mm}) \left( 30 - 10 \left( 300 \text{ mm} \times 30 \text{ mm} \right) \right) \]

\[ A_s = 589.5 \text{ mm}^2 \]

\[ A_y = 589.5 \text{ mm}^2 \times \text{ 15} \text{ kN/m} \times 10 \text{ kN/m} \times 0.0002 \text{ kN/mm} \]

\[ \text{Strength of beam} = 50 \text{ kN} \times \left( 1000 \text{ mm} \times 1000 \text{ mm} \right) = 50 \text{ kN} \]

\[ A_1 = 589 \text{ mm}^2 \]

\[ \text{Beam 150 x 200} \]

\[ \text{Beam 150 x 200} \]
\[ \begin{align*}
A_y &= \frac{A_s}{N} \\
&= \frac{200 \text{ mm}^2}{1000} \\
&= 0.2 \text{ mm} \\
&= 66.7 \text{ mm}^2 / 589 \text{ mm}^2 \\
\text{moment area of steel is greater than concrete.}
\end{align*} \]

1. \textit{Calculate that minimum required area is satisfied (ASCE 413.8.2.6.1)}

\[ \begin{align*}
P &= 0.85 \times 66.7 \text{ mm}^2 \\
&= 5.67 \times 10^{-3} \\
(\text{concrete}) \times 200 \text{ mm} &= 0.002345
\end{align*} \]

\[ f = 0.002345 < f_p = 0.022 \Rightarrow OK! \]

- Safe enough for stirrups calculation now.

2. \textit{Calculate that the minimum required reinforcements are satisfied (ACI 4.4.2.3.6 or 4.20.1)}

- Calculate the gross gross section area for the unit Design

\[ A_g = b \times h \\
&= 180 \times 300 \\
&= 54,000 \text{ mm}^2 \\
A_g &= 540 \text{ mm}^2 \]

- Determine minimum reinforcement area

\[ A_{sd} = 0.002 \times A_g \\
&= 0.002 \times 540 \text{ mm}^2 \\
A_{sd} &= 660 \text{ mm} \\
\text{area} \geq 667 \text{ mm}^2 \Rightarrow A_{sd} = 660 \text{ mm}^2 \Rightarrow OK! \]

\[ \begin{align*}
\text{Calculate } M_y \\
\text{Calculate the area of conventional steel}
\end{align*} \]

\[ \begin{align*}
A_y &= \frac{A_s}{N} \\
&= \frac{200 \text{ mm}^2}{1000} \\
&= 0.2 \text{ mm} \\
&= 66.7 \text{ mm}^2 / 589 \text{ mm}^2 \\
\text{moment area of steel is greater than concrete.}
\end{align*} \]

\[ \begin{align*}
\alpha &= \frac{200 \text{ mm}^2}{1000} \\
&= 0.85 \times 589 \text{ mm}^2 \\
(0.85) \times 200 \text{ mm}^2 \\
&= 0.05 (0.85)(200 \text{ mm}^2) \\
&= 12.9 \text{ mm}
\end{align*} \]

- Moment resistance is thus calculated as

\[ \begin{align*}
M_y &= \frac{(d - y)}{2} \\
&= \frac{(200 - 12.9)}{2} \\
&= (200/200 \text{ mm}) (280 - 12.9) \\
&= 56.98 \text{ kN} \text{m} \\
\text{Since } M_y = 57 \text{ kN} \text{m} \geq M_y = 56.98 \text{ kN} \text{m} \Rightarrow OK!
\end{align*} \]
- CHECK THE CRACK CONTROL PARAMETERS
  - **a)** DECREASE THE EFFECTIVE RANDOM AREA AS \( A_{Eh} = A_e \cdot k_{Eh} \)
- CALCULATE THE SPACING \( d_s \) FROM THE LEAST OF:
  - THE TRANSVERSE SPACING OF THE REINFORCEMENT: \( d_s = \frac{1}{2} A_e \cdot k_{Eh} \)
  - THE CLEARANCE BETWEEN THE CONCRETE BARS: \( \frac{d_s}{2} + d \leq 300 \) mm

\[
\begin{align*}
    d_s &= d_e = h - d \\
    &= \frac{3}{4}h - 300 \\
    &= 300 - 300 \\
    &= 0 \text{ mm}
\end{align*}
\]

- **b)** THEREFORE, NEAR EFFECTIVE AREA \( A_e \)
  \[
  A_e = 0.6 \times 290 \text{ mm}^2
  \]

\[
\begin{align*}
    A_e &= 290 \text{ mm}^2
\end{align*}
\]

- DETERMINE THE DRESS IN THE DIAGONAL REINFORCEMENT \( f_d' \)
  - UNDER THE SERVICE LOAD LEVEL:
    \[
    f_d = 0.6 f_{yd} = 0.6 \times 1.09 \text{ kN/mm}^2
    \]
    \[
    f_d = 290 \text{ N/mm}^2
    \]

- DETERMINE THE VALUE OF \( z \)
  \[
  z = f_c' \sqrt{\frac{A_e}{A_e - A}}
  \]
  \[
  200 \text{ N/mm}^2 \times \sqrt{\frac{290 \text{ mm}^2}{290 \text{ mm}^2 - 290 \text{ mm}^2}}
  \]
  \[
  z = 22.279 \text{ kN} \text{/mm}^2
  \]
  \[
  \text{SINCE} \ z = 22.279 \text{ kN/mm}^2 < 25000 \text{ kN/mm}^2 \]
  \[
  \text{SINCE} \ z = 22.279 \text{ kN/mm}^2 < 25000 \text{ kN/mm}^2 \]
  \[
  \text{THE CSA A23.3 CRACKING CONTROL REQUIREMENTS ARE SATISFIED.}
  \]

- DETERMINE STiffNESS AND Torsional Rigid:
  - \( f_{yd} = 415 \) kN/mm²
  - \( f_c' = 20 \) MPa
  - \( s_{min} = 420 \text{ mm} \)

- THE Maximum Bend Spacing, \( s_{max} \) is LEAST OF:
  - \( s_{min} = 420 \text{ mm} \)
  - \( s_{min} = 500 \text{ mm} \)

- Radiata Box Spacing:
  \[
  r \leq d_t / 1000
  \]
  \[
  r \leq 20 \text{ mm}
  \]
  \[
  r \leq 200 \text{ mm}^2
  \]
  \[
  r \leq 600 \text{ mm}^2
  \]
  \[
  r \leq 333 \text{ kN} \text{/mm}^2
  \]

- Required: use 15 mm 360 for Intermediate beam

- Placed 300 temp anch.
Check the slab design for shear.

Find the max shear force at quarter:

\[ V_f = \frac{W_f \times L}{2} = \frac{11 \times 4}{2} = 22 \text{ kN} \]

\[ V_f = 33 \text{ kN} \]

Find effective depth:

\[ d_e = 0.7 - \frac{d_e}{0.7} = 0.7 - \frac{15}{2} = 25.5 \]

Find effective shear depth dy:

\[ dy = 0.9 (302) = 272.8 \text{ mm} \]

\[ dy = 372 \text{ mm} \]

Check dy = 270.75 mm approx.

Nominal \( f_d \) is 11.3.6.2 for slab thickness not greater than 350 mm. 30-321 can be used.

Determine \( V_c \):

\[ V_c = \frac{0.85}{1.1} \times \frac{102}{1.1} \times \frac{100}{2} \times \frac{30}{1000} = 155.4 \text{ kN/m} \]

Since \( V_c = 155.4 \text{ kN} > V_f = 33 \text{ kN} \)

\[ \text{shear design is not adequate.} \]
Check for overturning: (Min FOS: 2-3)

\[
W = 8 \times A = 69.7 \text{ kN}
\]

\[
\begin{align*}
W_3 &= 2 \times A = 29.2 \text{ kN} \\
W_3 &= -36 \text{ kN}
\end{align*}
\]

\[
\begin{align*}
\Sigma V &= W = 69.7 \text{ kN} \\
\Sigma R &= 0
\end{align*}
\]

\[
\begin{align*}
R_a &= \frac{1}{2} k_a 8 H^2 = 42.0 \text{ kN/m} \\
P_p &= 4.14 \text{ kN/m}
\end{align*}
\]

\[
\begin{align*}
\Rightarrow \ FOS_{\text{overturning}} &= \frac{\Sigma M_R}{M_0} \\
\Sigma M_R &= (4.675m)(W_1) + (\frac{1}{3})(0.3m)P_p + (4.1m)(V_2) + (4.1m)(V_3) = 572.8 \text{ kN.m}
\end{align*}
\]

\[
M_0 = \frac{H}{3} P_p = 61.6 \text{ kN.m}
\]

\[
\Rightarrow \ FOS_{\text{overturning}} = 9.6 \ (> \ FOS_{\text{min}} = 2)
\]

Check for sliding: (Min FOS: 1.5)  \( S = 26^\circ \) (NAVFAC Standards)  \( \ast \) Passive Force Ignored  \( \ast \) Adhesion Ignored

\[
W_6 = \text{Weight of Sand}; \ W_c = \text{Weight of Concrete}
\]

\[
\begin{align*}
\Sigma V &= W_6 + W_c = 134.7 \text{ kN} \\
\Sigma R &= \frac{V_6 \tan \theta + P_6}{\beta_a} = 1.56 \ (> FOS_{\text{min}} = 1.5)
\end{align*}
\]
\[ X = \frac{M_{\text{tot}}}{\Sigma V} = 3.94 \text{ m} \quad c = 4.25 \text{ m} - \bar{x} = 0.31 \left( \frac{B}{6} \right) \]

\[ q_{\text{max}} = q_{\text{ue}} = \frac{\Sigma V}{B} \left( 1 + \frac{c}{B} \right) = 36 \text{ kN/m} \quad q_{\text{min}} = q_{\text{heel}} = 22.9 \text{ kN/m} \]

\[ q_{\mu} = 0.6N_{\mu} \sum F_i + q_{\mu} N \sum F_i + \frac{1}{2} \epsilon \phi N \sum F_i \sum F_i \]

where:

\[ \phi = 80 = 6^\circ \quad B = 0.300 \text{ m} \quad \epsilon = \tan \left( \frac{\phi}{2} \right) = 86.8^\circ \]

\[ B' = B - 2e = 4.48 \quad F_{d} = 1 + 0.4 \left( \frac{B}{B'} \right) = 1.027 \]

\[ F_{d} = 1 + 2 \tan(\phi)(1 - \sin(\phi))\left( \frac{B}{B'} \right) = 1.01 \]

\[ F_{d} = 1 \quad F_{d}^{i} = F_{i} = (1 - \frac{\phi}{90^\circ}) = 0.0013 \quad F_{8i} = (1 - \frac{\phi}{90^\circ}) = 0.0013 \]

\[ N_{q} = \left[ e^{(\frac{45 + 50}{2})} \right] = 64.2^\circ \quad N_{q} = (N_{q} - 1) \tan(1.4\phi') \]

\[ q_{\mu} = 5751 \text{ kN} \Rightarrow FOS_{\text{bending}} = \frac{q_{\mu}}{q_{\mu}} = \frac{5751}{36} = 160 \Rightarrow FOS_{\text{min}} = 3 \]
APPENDIX D: DETAILED DESIGN DRAWINGS

D.1 CONCRETE CONSTRUCTION
CONCRETE FORM WORK
1. ALL FORM WORK IS TO BE SELECTED BY THE CONTRACTOR TO MEET THE DIMENSION SPECIFICATIONS FOR CONCRETE REINFORCEMENT AND FINAL CONCRETE DIMENSIONS AS SHOWN IN DWG 445-CIVL-13 SHEETS C-01, C-02, AND C-03. THE FABRICATION AND ERECTION OF THE FORM WORK MUST BE APPROVED BY THE FIELD ENGINEER OR ENGINEERS REPRESENTATIVE AND CONSTRUCTED IN ACCORDANCE WITH CSA-S240.3-14.

STEEL REINFORCEMENT
1. ALL STEEL REINFORCEMENT TO BE DONE IN REFERENCE TO DWG 445-CIVL-13 SHEETS C-01, C-02, AND C-03. STEEL REINFORCEMENT WORKS SHALL BE APPROVED BY THE ENGINEER OR ENGINEERS REPRESENTATIVE BEFORE CONCRETE POURS ARE COMMENCED.
3. REINFORCEMENT LAYOUT IN EACH ELEMENT OF THE SECTIONS ARE SHOWN IN DWG 445-CIVL-13 C-02 AND C-03.

CONCRETE
1. ALL CONCRETE WORK IS TO BE DONE IN ACCORDANCE WITH CSA A23.3 DESIGN CODE REQUIREMENTS.
2. BASE STRUCTURE AND RETAINING WALL ARE PLANNED TO BE POURED DURING THE SAME POURING TO FORM A MONOLITHIC BASE AND RETAINING CONCRETE STRUCTURE.
3. THE SLAB SYSTEM WILL BE POURED IN FORM ON LEVEL GROUND ADJACENT TO THE BASE. AFTER HARDENING IT WILL BE CONNECTED AT THE TOP OF THE RETAINING WALL AND AT THE END OF THE BASE.
4. TRACK SURFACE WILL BE SEALED TO PREVENT SEEPAGE.

GENERAL
1. IMPORTANT MILESTONES DURING CONSTRUCTION MUST HAVE THE APPROVAL OF A DESIGNATED ENGINEER WHO HAS CONDUCTED A FIELD VISIT AND INSPECTION.
2. ALL MATERIAL WASTES MUST BE HANDLED ACCORDING TO THE ENVIRONMENTAL MANAGEMENT PLAN.
3. CONSTRUCTION WORKS MUST BE COMPLETED IN SEQUENCE WITH THE WINS AND SCHEDULE SUBMITTED WITH THIS DRAWING SET.
CONCRETE FORM WORK

1. All form work is to be selected by the contractor to meet the dimensions specified. Concrete reinforcement and final concrete dimensions shown in DWG 445-CIVL-13 sheets C-01, C-02 and C-03.

2. The fabrication and erection of the form work must be approved by the field engineer or engineer representative and constructed in accordance with CSA A23.3-14.

STEEL REINFORCEMENT

1. All steel reinforcement to be done in reference to DWG 445-CIVL-13 sheets C-01, C-02 and C-03 dimensions and in accordance with CSA A23.3-14.

2. Steel reinforcement works shall be approved by the engineer or engineer representative before the concrete pours are commenced.

3. Reinforcement layout in each element of the sections are shown in DWG 445-CIVL-13 sheets C-02 and C-03.

CONCRETE

1. All concrete work is to be done in accordance with CSA A23.3 design code requirements.

2. Base structure and retaining wall are planned to be poured during the same pouring to form a monolithic base and retaining concrete structure.

3. The slab system will be poured in form on level ground adjacent to the base. After hardening it will be connected at the top of the retaining wall and at the end of the base.

4. Track surface will be sealed to prevent seepage.

ISSUED FOR CONSTRUCTION
CONCRETE FORM WORK

1. All concrete work is to be selected by the contractor to meet the dimension specifications for concrete reinforcement and final concrete dimensions as shown in DWG 445-CIVIL-13 sheets C-01, C-02, and C-03 dimensions and in accordance with CSA-A23.3-94 design code requirements.

2. The fabrication and erection of the form work must be approved by the field engineer or engineers representative and constructed in accordance with CSA-A23.3-94 design code requirements.

STEEL REINFORCEMENT

1. All steel reinforcement to be done in reference to DWG 445-CIVIL-13 sheets C-01, C-02, and C-03 dimensions and in accordance with CSA-A23.3-94 design code requirements.

2. Steel reinforcement works shall be approved by the engineer or engineers representative before concrete pours are commenced.

3. Reinforcement layout in each element of the sections are shown in DWG 445-CIVIL-13 C-02 and C-03.

GENERAL

1. Important milestones during construction must have the approval of a designated engineer who has conducted a field visit and inspection.

2. All material wastes must be handled according to the environmental management plan.

3. Construction works must be completed in sequence with the WBS and schedule submitted with this drawing set.
D.2 SITE PLANS
RAIN GARDEN CROSS SECTION

RAIN GARDEN - PLAN VIEW

ALL DIMENSIONS IN METERS

PRODUCED BY AN AUTODESK STUDENT VERSION
Appendix E: Artist Renderings of the Facility